

# Arkansas River Corridor Projects

## Existing Geotechnical Resource Review

TO: Tulsa County

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FROM: CH2M HILL

DATE: November 19, 2009

PROJECT NUMBER: 386594

Tulsa County, as part of the Arkansas River Corridor Master Plan (Carter & Burgess, 2004; C. H. Guernsey and Company et al., 2005), is undertaking an improvement project on the Arkansas River. The primary goals of the overall project are to improve least tern habitat, improve fish habitat and fish passage, improve the function of the river system itself, enhance economic development, increase recreational opportunities, and increase connectivity between the river and surrounding communities. The conceptual project components are described in detail in the Technical Memorandum (TM) entitled Baseline Project Summary for the Arkansas River Corridor Project (CH2M HILL, 2009). Key components include:

- Design of habitat improvements along the corridor
- Design of bank stabilization in select areas
- Design of a new Sand Springs low-head dam, pedestrian bridge, and amenities
- Design of modifications to Zink Dam and lake with whitewater features
- Design of a new South Tulsa/Jenks low-head dam, pedestrian bridge, and amenities

This TM provides a summary of the existing geotechnical and geological resources for the Arkansas River Corridor Project, specifically near the three low-water dam sites (Sand Springs, Zink Lake, and South Tulsa/Jenks). The purpose is to develop an understanding of the general subsurface conditions in the vicinity of the project, and to identify gaps and data needs for future geotechnical efforts. The information sources reviewed, and a brief summary of the observations, are provided herein.

## Background and Site Location

As part of a master plan for the Arkansas River corridor, Tulsa County is considering a series of improvement projects on the Arkansas River, along a corridor stretching nearly 42 miles from Keystone Dam to the Tulsa County/Wagoner County border. Included in this corridor are three dam sites:

1. Sand Springs (proposed)

2. Zink Lake (upgraded)
3. South Tulsa/Jenks Dam (proposed)

Existing geologic literature and engineering reports were reviewed in order to summarize existing geotechnical subsurface information at these sites.

## Literature Review

Existing geologic mapping has been reviewed and summarized in the figures in Attachment A (Surficial Geology, Maps A through O). The data for these maps were taken from available literature and mapping archived with the U.S. Geological Survey for the Enid, Tulsa, Oklahoma City, and Fort Smith Quadrangles (USGS, 2009).

In addition, existing geotechnical information from five available sources in the project vicinity was reviewed. These five information sources are synopsized below.

### Reference 1—Stantec, Near Sand Springs and South Tulsa/Jenks

Stantec Consulting Services. Geotechnical Investigation and Testing; Arkansas River Corridor Project, Arkansas River. Prepared for the USACE Tulsa District, Tulsa, Oklahoma. Contract No. DACW912BV-07-D-1000. Sand Springs/Jenks, Oklahoma. May 2008.

### Exploration Program

Eight borings were drilled by Stantec Consulting Services, under contract to the U.S. Army Corps of Engineers (USACE), in March 2008. Five of these borings (J1-J5) were advanced at the proposed South Tulsa/Jenks dam location; the other three borings (S1, S2, and S4) were advanced at the proposed Sand Springs dam location. The approximate locations of the borings advanced by Stantec are shown on the figures in Attachment B. The borings were advanced to depths ranging from 10.5 to 75 feet (ft) below ground surface (bgs). Subsurface conditions at both locations generally consisted of sandy overburden (recent alluvium) overlying shale bedrock. At one location (S2), a thin layer (< 2 ft) of sandstone was encountered overlying the shale.

### Overburden

The thickness of the overburden ranged from approximately 4 to 10 ft within the river channel, and from 18 to 35 ft in the river banks. This material consisted of poorly graded sand to silty sand, with infrequent clay seams and low gravel content. The consistency of the overburden was typically very loose to loose, and medium dense at a few locations.

Laboratory tests performed on overburden samples included Atterberg limits tests and gradation tests.

### Bedrock

Each boring was terminated in shale bedrock. This material is gray to dark gray, thinly bedded, and moderately weathered and fractured in zones. The rock quality designation (RQD) was determined for the rock. In the South Tulsa/Jenks borings, the average RQD ranged from 46 to 86 percent, which corresponds to a poor to good rock mass (Deere and

Deere, 1989). In the three Sand Springs borings, the RQD ranged from 42 to 57 percent, which corresponds to a poor to fair rock mass.

Unconfined compression tests were conducted on rock core samples (15 tests performed). The average unconfined compressive strength in the South Tulsa/Jenks borings (9 tests) was 737 pounds per square inch (psi). In the Sand Springs borings (6 tests), the average unconfined compressive strength was 565 psi. This range in unconfined compressive strength indicates very weak rock with an average hardness of R1 (Deere and Miller [1966], Piteau [1971], and Robertson [1971]).

## Reference 2 – Kleinfelder, Near Jenks

Kleinfelder. Geotechnical Exploration--Confidential. Jenks, Oklahoma. August 2008.

### Exploration Program

Eight borings were drilled and logged by Kleinfelder, Inc. in August 2008 (B1-B8), near Jenks. The borings were advanced to depths ranging from 19 to 29 ft bgs, all along the right river bank of the Arkansas River. Subsurface conditions generally consisted of sandy overburden (recent alluvium) overlying shale bedrock.

### Overburden

The thickness of the overburden ranged from approximately 5.5 to 19 ft. This material consisted of very loose to medium dense poorly graded sand, with low gravel content. In a few locations, a thin (1- to 2.4-ft thick) layer of stiff lean clay was encountered interbedded within the sandy overburden material. No laboratory test results from samples of the overburden material were reported.

### Bedrock

Each boring was terminated in shale bedrock. This material is characterized as light gray to dark gray, soft to moderately hard, and moderately weathered in thin zones in some borings. No rock quality evaluation or laboratory testing was performed on the rock.

## Reference 3—State of Oklahoma Department of Transportation (ODOT), Near Bixby

State of Oklahoma, Department of Transportation. U.S. Highway No. 64, Memorial Drive. Design Plans for a bridge replacement (Bridge A) and subsurface information, near Bixby, OK. September 2002.

### Exploration Program

Borings were advanced in 2002 (B-1, -2, -4, and -5) at each corner of the proposed bridge, near Bixby (see Attachment C). All four of these borings were located away from the Arkansas River channel, just over a mile west of the river bank. The four borings were advanced to depths ranging from 50 to 54 ft bgs. Subsurface conditions generally consisted of interbedded clay and sand (recent alluvium) overlying highly weathered gray shale.

## Overburden

The thickness of the overburden ranged from approximately 20 to 23.3 ft. This material consisted of approximately 8 to 23 ft of very soft to soft lean to fat clay, overlying sand. No laboratory test results were reported in the overburden material.

## Bedrock

Each boring was terminated in shale bedrock. This material is characterized as gray, dry to wet, and unweathered to highly weathered. Texas Cone Penetrometer (TCP) testing was performed at regular intervals within the rock. These test values ranged from ½ inch to over 2 inches per 100 blows. No rock quality evaluation or laboratory testing was reported.

## Reference 4—ODOT, Near Jenks

State of Oklahoma, Department of Transportation. Arkansas River Bridge and West Approach: City of Jenks. East Approach: City of Tulsa. Design Plans, near Tulsa, OK. July 1994.

## Exploration Program

Twenty-two borings were advanced in 1994 adjacent to the existing bridge piers across the Arkansas River near Jenks (see Attachment D). The borings were advanced to depths ranging from 24.1 to 55 ft bgs. Subsurface conditions generally consisted of sand within the river bottom (recent alluvium) overlying weathered shale.

## Overburden

The thickness of the overburden ranged from approximately 6 to 9.5 ft within the Arkansas River bottom, and increased to between 20 and 31 ft on the river banks. The predominant overburden material within the river bottom is sand. In the borings along the river banks, the sand overburden is interbedded with silt and clay layers of varying thickness. No laboratory test results were reported in the overburden material.

## Bedrock

Each boring was terminated in unweathered to weathered shale bedrock. TCP testing was performed periodically in some borings within the rock. These test values ranged from ¼ inch to 1.5 inches per 100 blows. No rock quality evaluation or laboratory testing was reported.

## Reference 5—W.R. Holway & Associates. Tulsa River Parks Authority, at Zink Lake Dam

W.R. Holway & Associates. Tulsa River Parks Authority, Low Water Dam Project: A Project of the Tulsa Urban Renewal Authority. Record Drawing for Zink Lake Low Water Dam, near Tulsa, OK. June 1983.

## Exploration Program

Ten borings were advanced in 1980 (Core Holes A through H, and Core Holes #2 and #8), along the alignment of the Zink Lake Dam (see Attachment E). Two borings were advanced near each abutment, and six borings were advanced within the Arkansas River channel. The

borings were advanced to depths ranging from 55.1 to 84.6 ft bgs. Subsurface conditions generally consisted of sand, silt, and clay (recent alluvium) overlying sandstone, overlying shale. The foundation drawing indicates boring logs are available, though they were not obtained as part of this review.

### Overburden

The thickness of the overburden ranged from approximately 2 to 10 ft within the Arkansas River bottom, and increased to between 23 and 28 ft on the river banks. This material was described as sand, silt, and clay. No description of bedding sequence or interbed layer thickness was available. No laboratory test results from samples of the overburden material were reported.

### Sandstone

The thickness of the sandstone was observed to be relatively consistent in the 10 borings, ranging from 31 to 39 ft over the length of the Zink Lake Dam. The elevation of the sandstone surface was also relatively consistent, and was encountered between El. 603 and 610. The surface of the sandstone was actually encountered higher in elevation beneath the river channel than beneath the abutments on the river bank. No information on rock quality, degree of weathering, jointing or fracture patterns, or hardness was available.

### Bedrock (Shale)

Each boring was terminated in shale bedrock. The elevation of the shale surface ranged from El. 564 to 579, and the borings indicate the surface of the shale gradually dips from east to west. No information on rock quality, degree of weathering, jointing or fracture patterns, or hardness was reported.

## General Subsurface Conditions

Based on conditions reviewed and summarized above, the subsurface conditions are anticipated to be relatively consistent near the Arkansas River in the project vicinity. The typical subsurface conditions documented in existing reports and bridge investigations include interbedded silt, clay, and sand overburden overlying predominantly shale bedrock. The stick logs from borings advanced at the Zink Lake Dam indicate a layer of sandstone overlying the shale bedrock at this location. The sand-dominated overburden was observed to have a consistency of very loose to medium dense. Layers of clay were observed to range from lean clay to fat clay, with a consistency of very soft to soft. Groundwater elevations along the river banks are presumably similar to the adjacent river level.

The shale bedrock in the project vicinity is massive and was encountered in every boring location reviewed for this TM. Typically, it is encountered at depths ranging from 5 to 10 ft bgs within the river bottom, to between 20 and 30 ft along the river banks. The shale is identified as part of several geologic formations, including the Nellie Bly formation, the Wewoka formation, and the Senora formation. The shale is described as gray, thinly bedded, and highly weathered in zones (independent from depth below surface). Based on limited TCP testing within the shale, it is estimated to have a typical allowable point bearing greater than 30 tons/square foot (~420 psi).

In recent borings advanced at the Sand Springs and South Tulsa/Jenks areas (Stantec, 2008), RQD and unconfined compressive strength were evaluated at each location. Near the Sand Springs location, the average RQD of the shale was observed to be 48 percent, based on findings from 3 borings. The average unconfined compressive strength of tested specimens was 565 psi. At the South Tulsa/Jenks location, the average RQD of the shale was observed to be 72 percent. The average tested unconfined compressive strength of shale was 737 psi.

## Key Concerns and Data Gaps

The depth and properties of the bedrock, based on the limited information reviewed, indicate that the material would provide a suitable foundation for the proposed dams and improvements. However, there is still key information that needs to be obtained in order to minimize the potential geotechnical risk at each of the sites. The primary concerns at the three dam sites include identifying the erosion and seepage potential of the shale rock upon which the dams will be constructed, determining the depth of weathering, and identifying the locations of possible ancient river channels in the bedrock surface.

Additional geotechnical information on consistency, strength, and grain size should be collected within the overburden at both proposed dam sites (Sand Springs and South Tulsa/Jenks), and at the existing Zink Lake Dam site. Standard Penetration Tests (SPTs) should be conducted at regular intervals in boreholes advanced within the overburden. The thickness of the overburden and depth to rock should also be more closely examined along specific alignments of the proposed dams.

At the Sand Springs, South Tulsa/Jenks, and Zink Lake dam sites, additional strength data and characterization of the bedrock are necessary both in the river channel and along the banks in order to determine specific rock mass properties and guide final selection of dam locations. Borings should be advanced into rock using rock coring methods, in order to determine rock quality, degree of weathering, durability, and strength as a function of depth. The general type and extent of recommended geotechnical exploration will be outlined in a separate TM by CH2M HILL (Arkansas River Corridor Projects: Preliminary Geotechnical Exploration).

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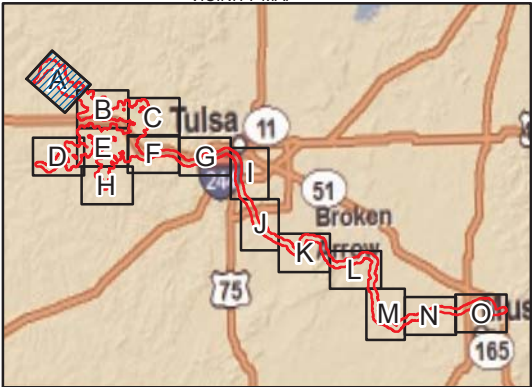
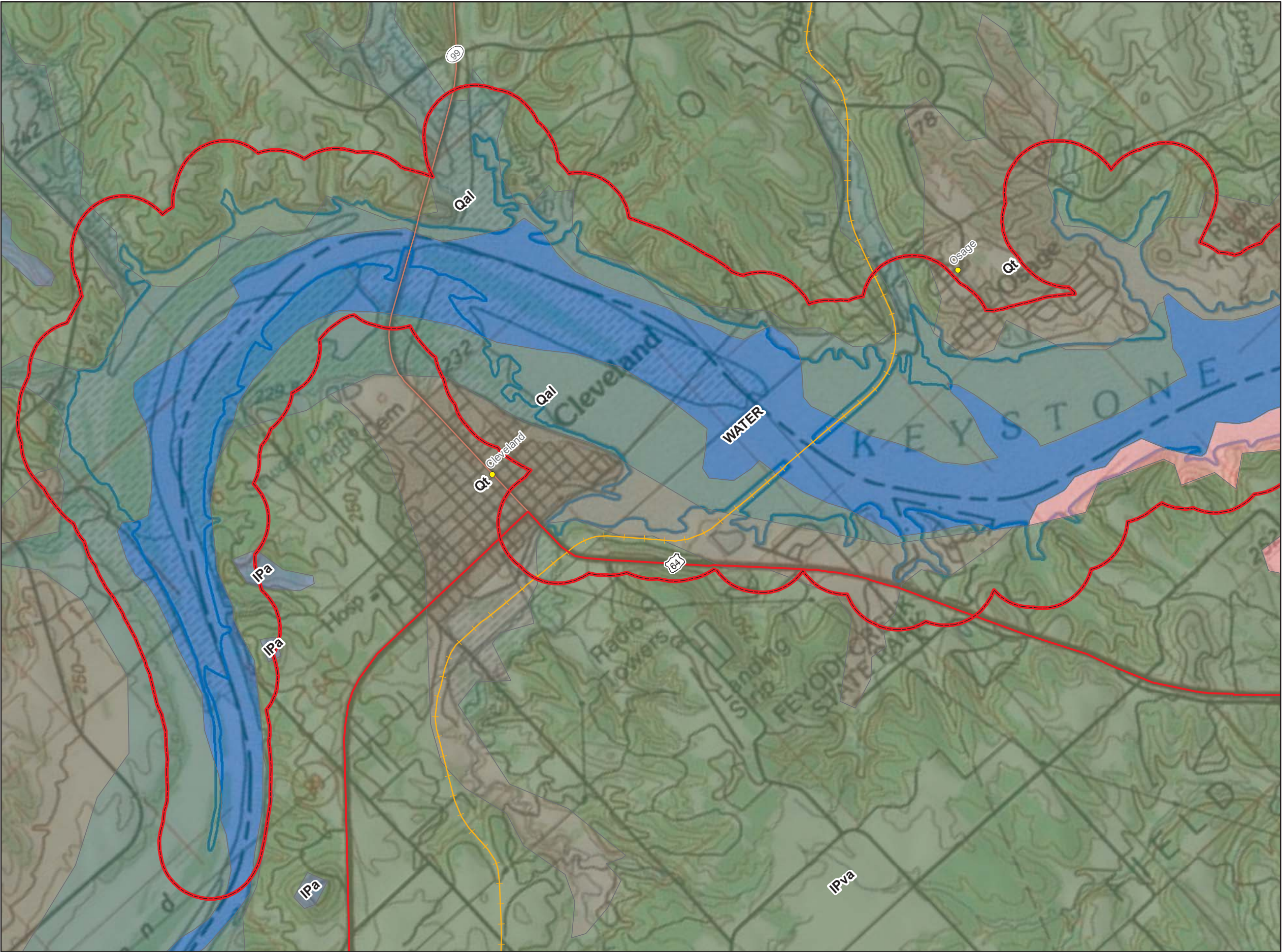
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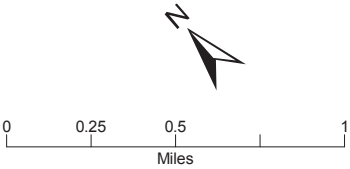




LEGEND

- Existing Dam
- Proposed Dam
- Arkansas River Buffer
- Interstates
- US & State Highways
- Secondary State & County Roads
- Railroads
- Keystone Lake

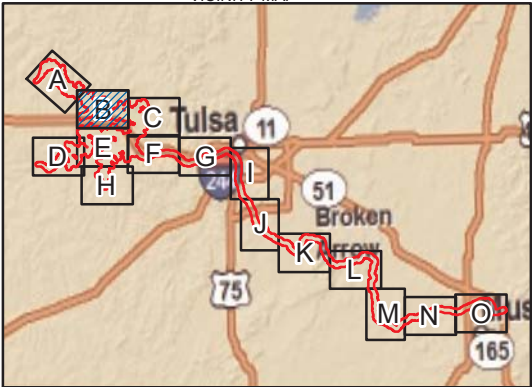
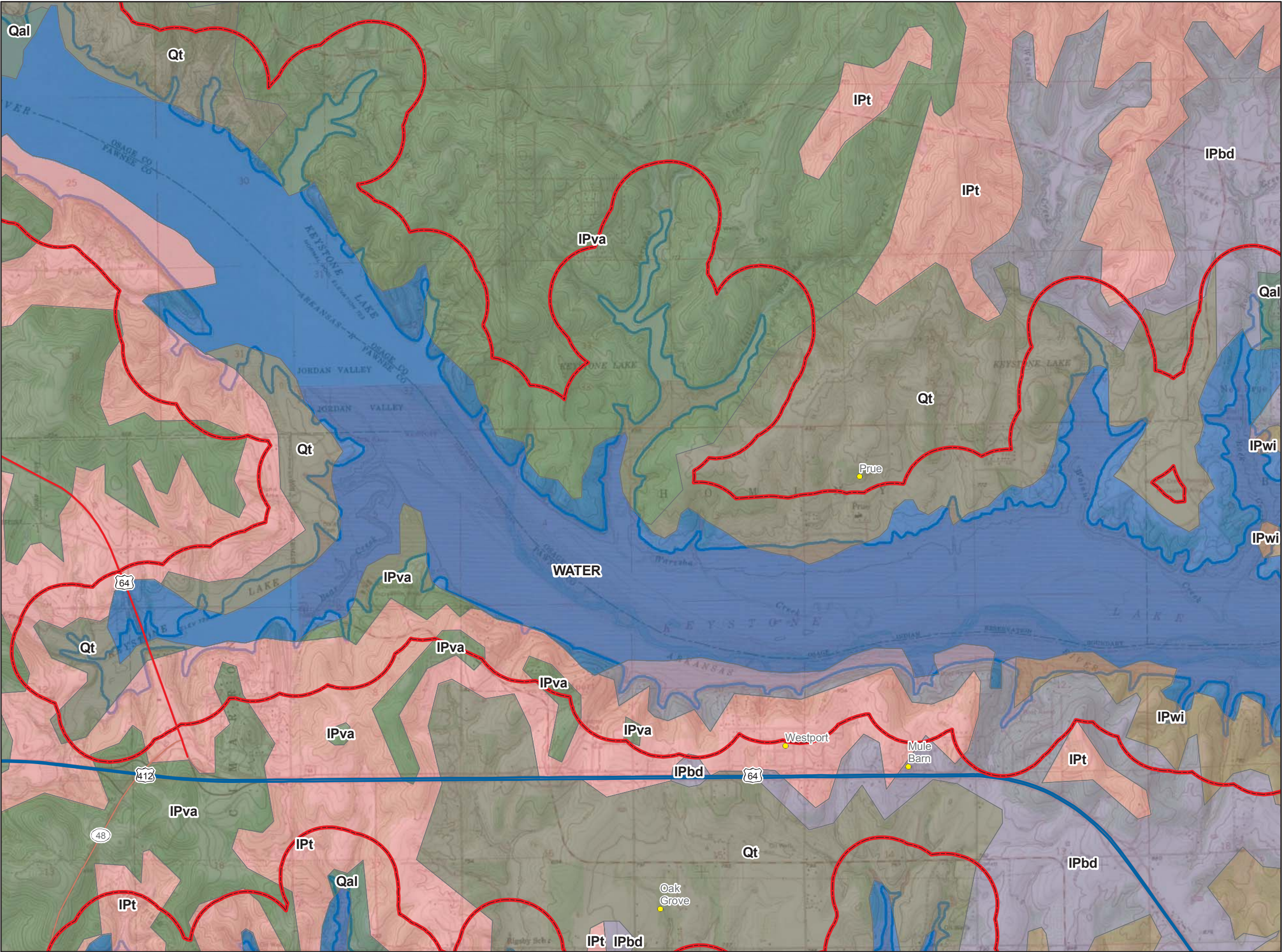
Symbol	Name
Qal	Alluvium
Qt	Terrace Deposits
IPva	Vamoosa Formation
IPt	Tallant Formation / (Thurman Sandstone)
IPbd	Barnsdall Formation
IPwi	Wann and Iola Formations
IPa	Atoka Formation
IPch	Chanute Formation
IPnh	Nellie Bly Formation and Hogshooter Limestone
IPd	Dewey Formation
IPh	Hogshooter Formation / (Holdenville Shale)
IPcc	Coffeyville and Checkerboard Formations
IPsl	Seminole Formation
IPhl	Holdenville and Lenapah Formations
IPnw	Nowata Formation
IPhd	Holdenville Shale
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IPsn	Senora Formation
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IPsm	Savanna, McAlester, and Hartshorne Formations
IPsv	Savanna Formation



**Surficial Geology**  
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**Map A**

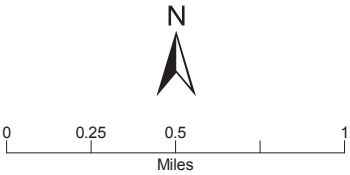




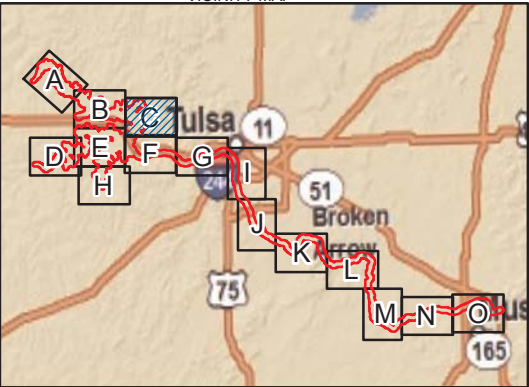
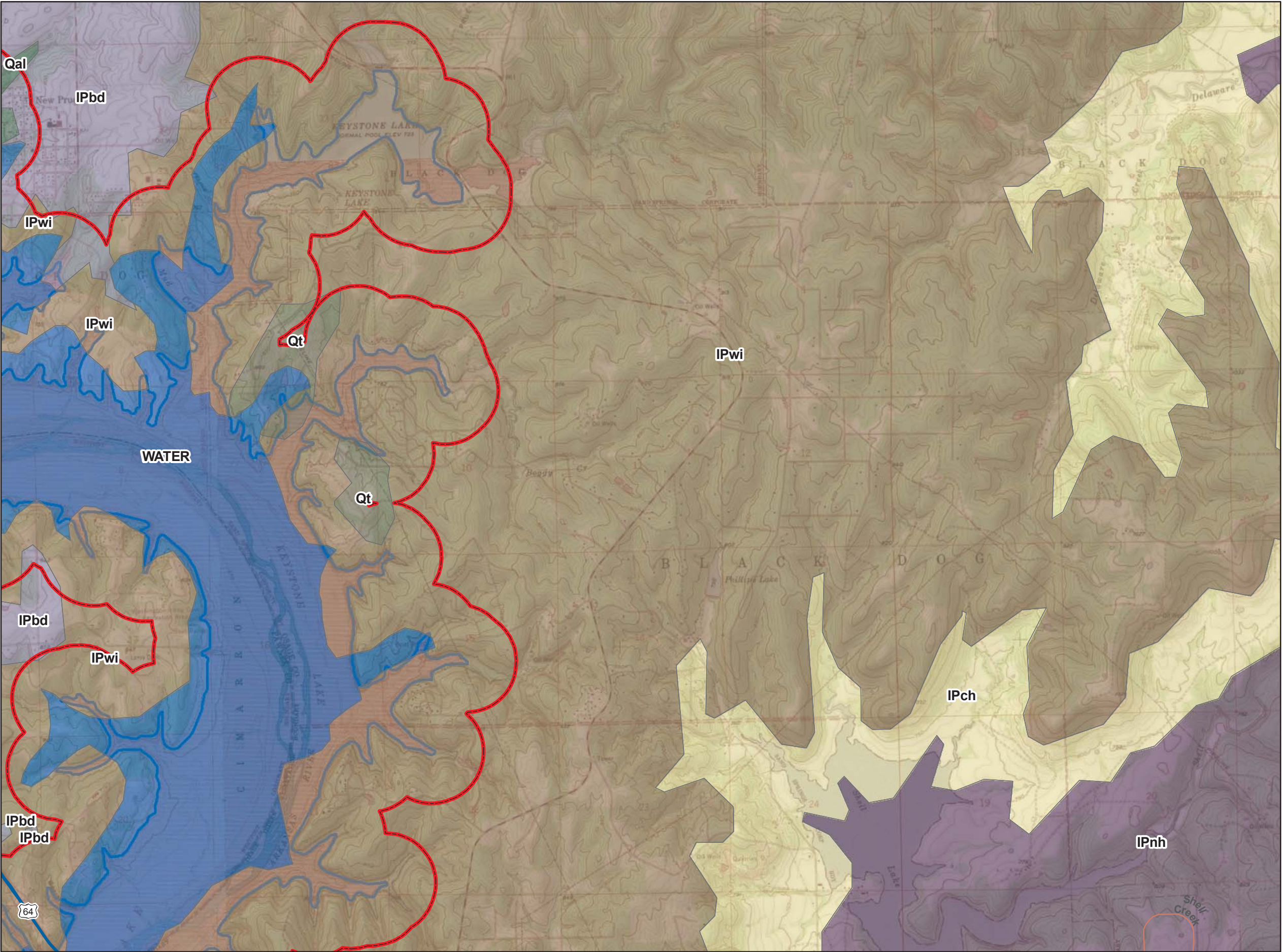
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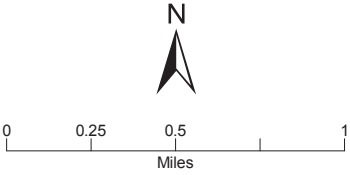




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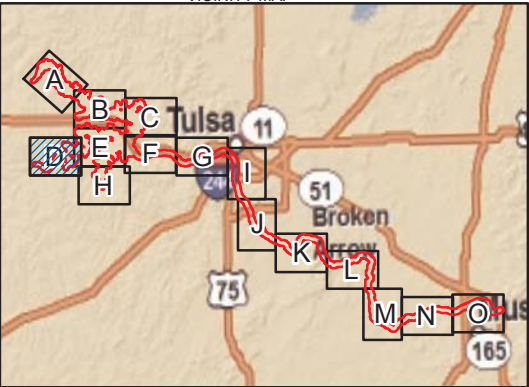
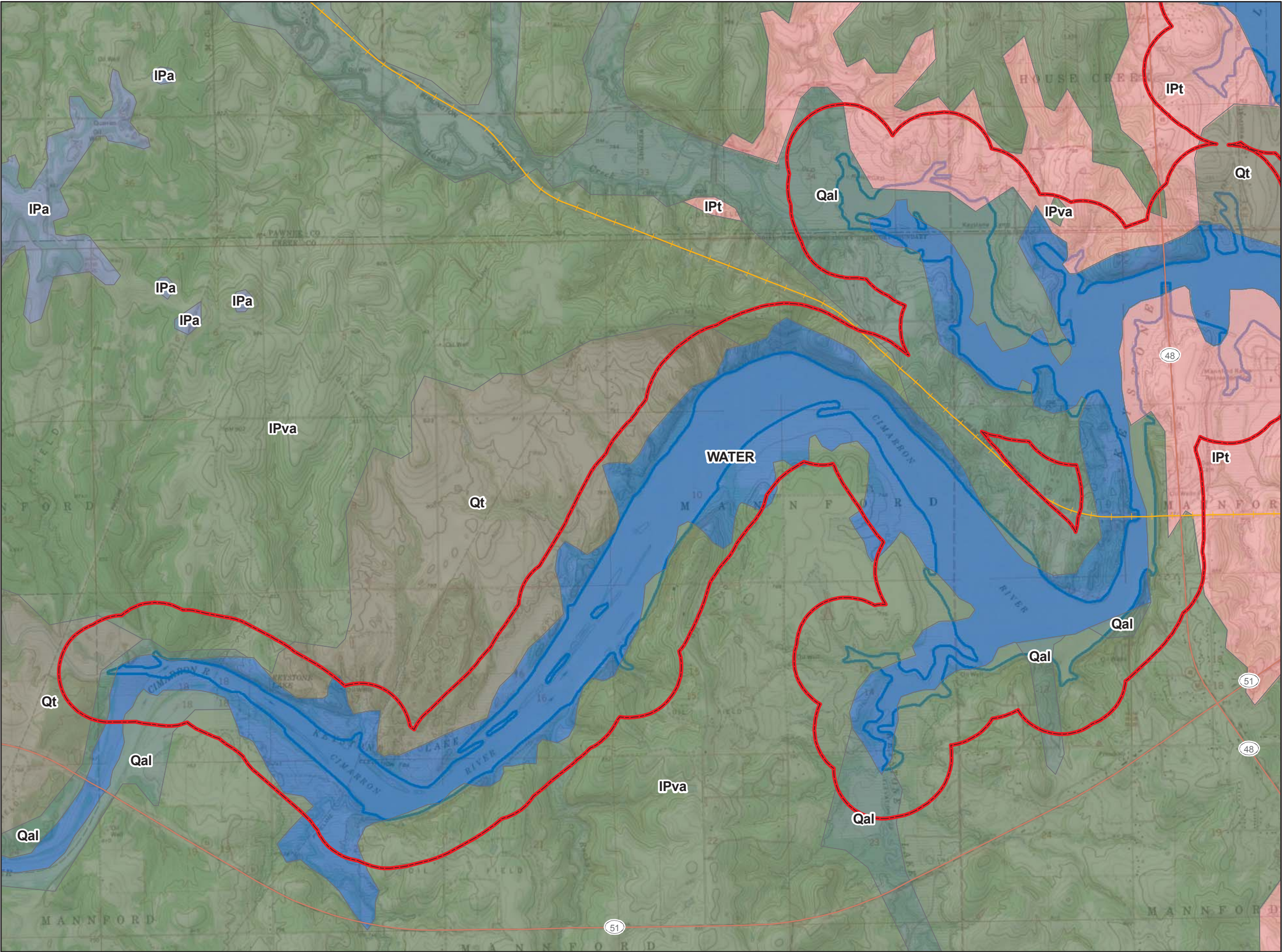
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**Map C**

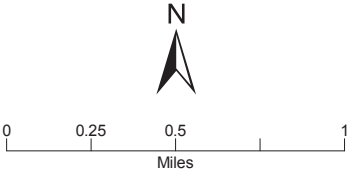




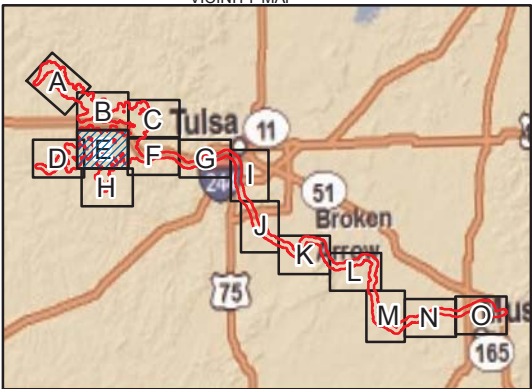
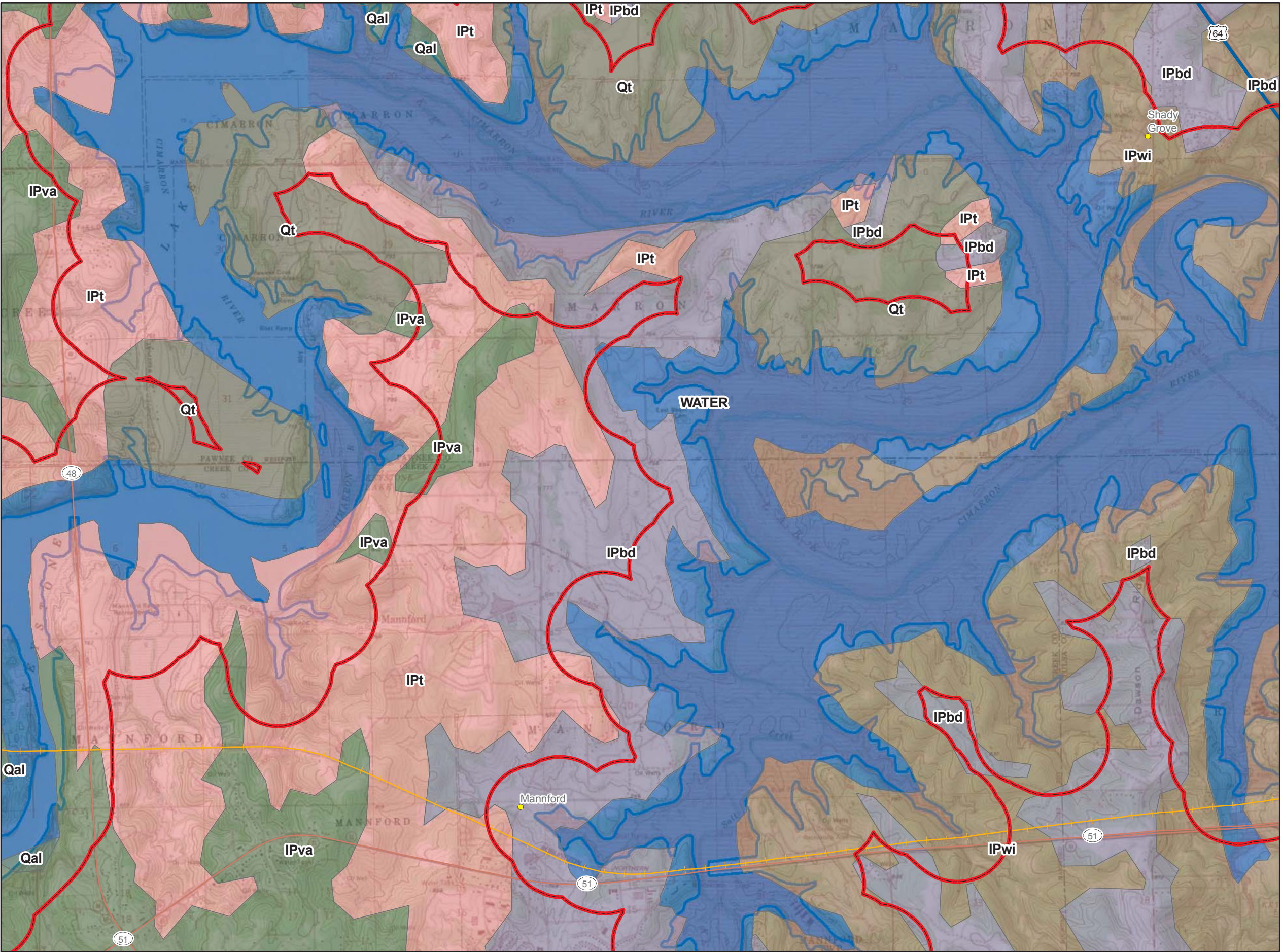
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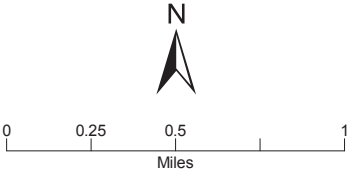




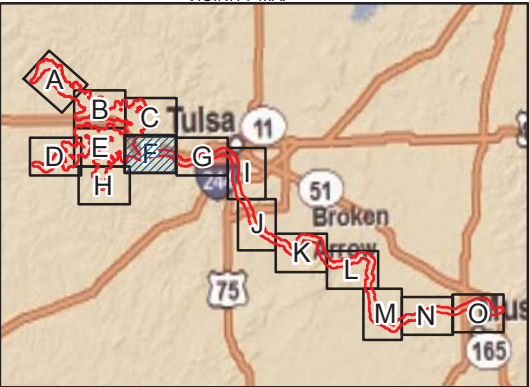
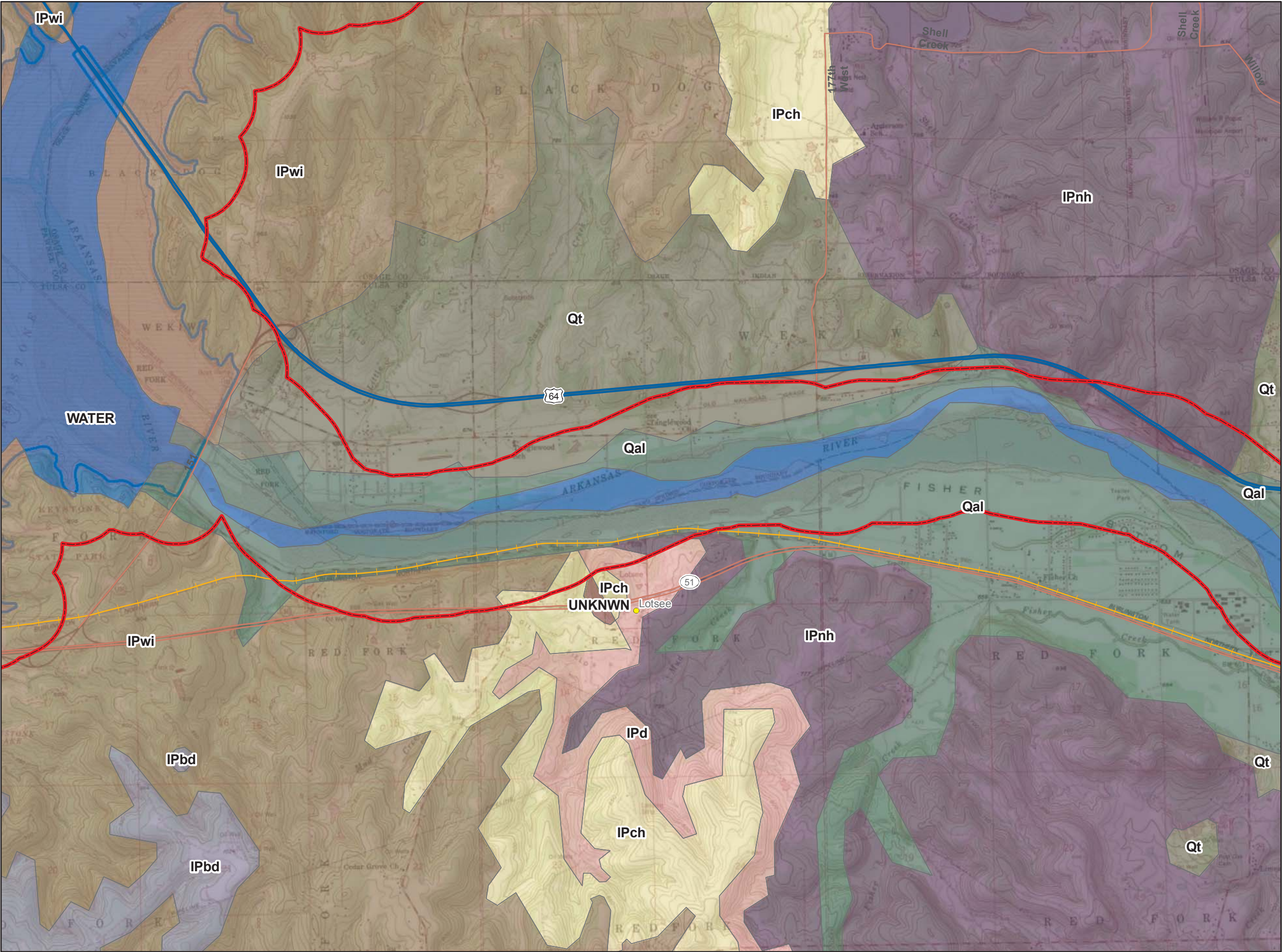
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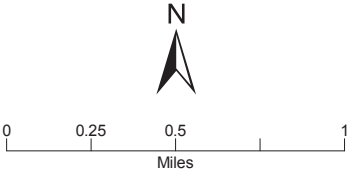




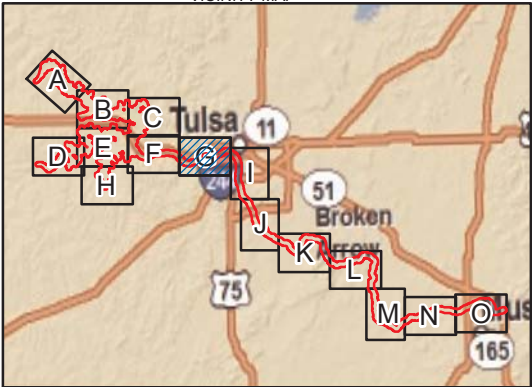
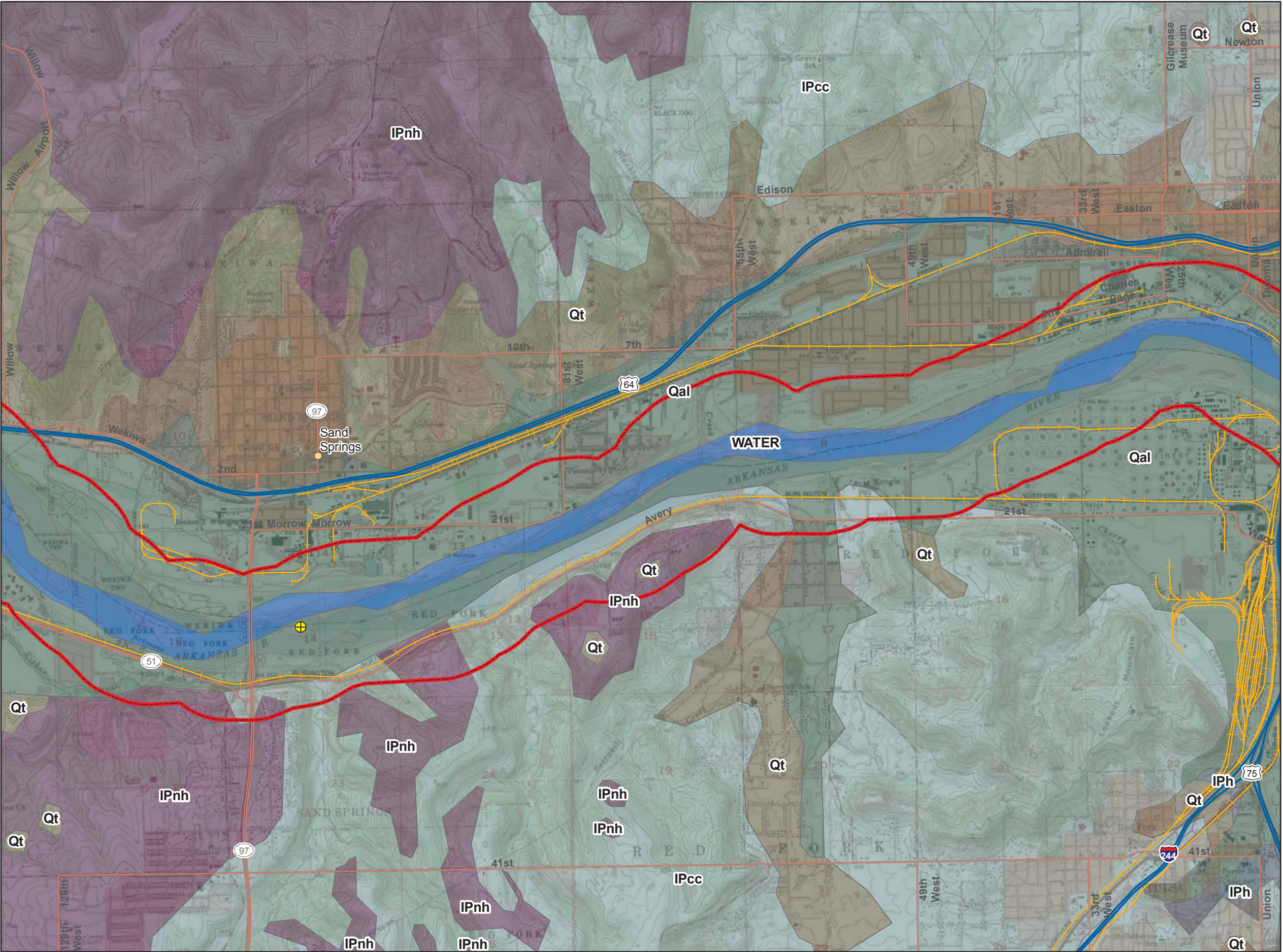
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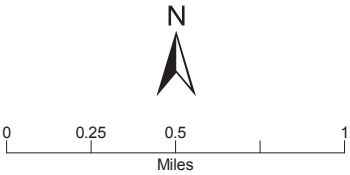




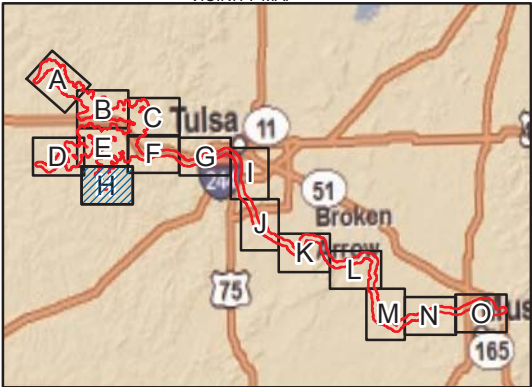
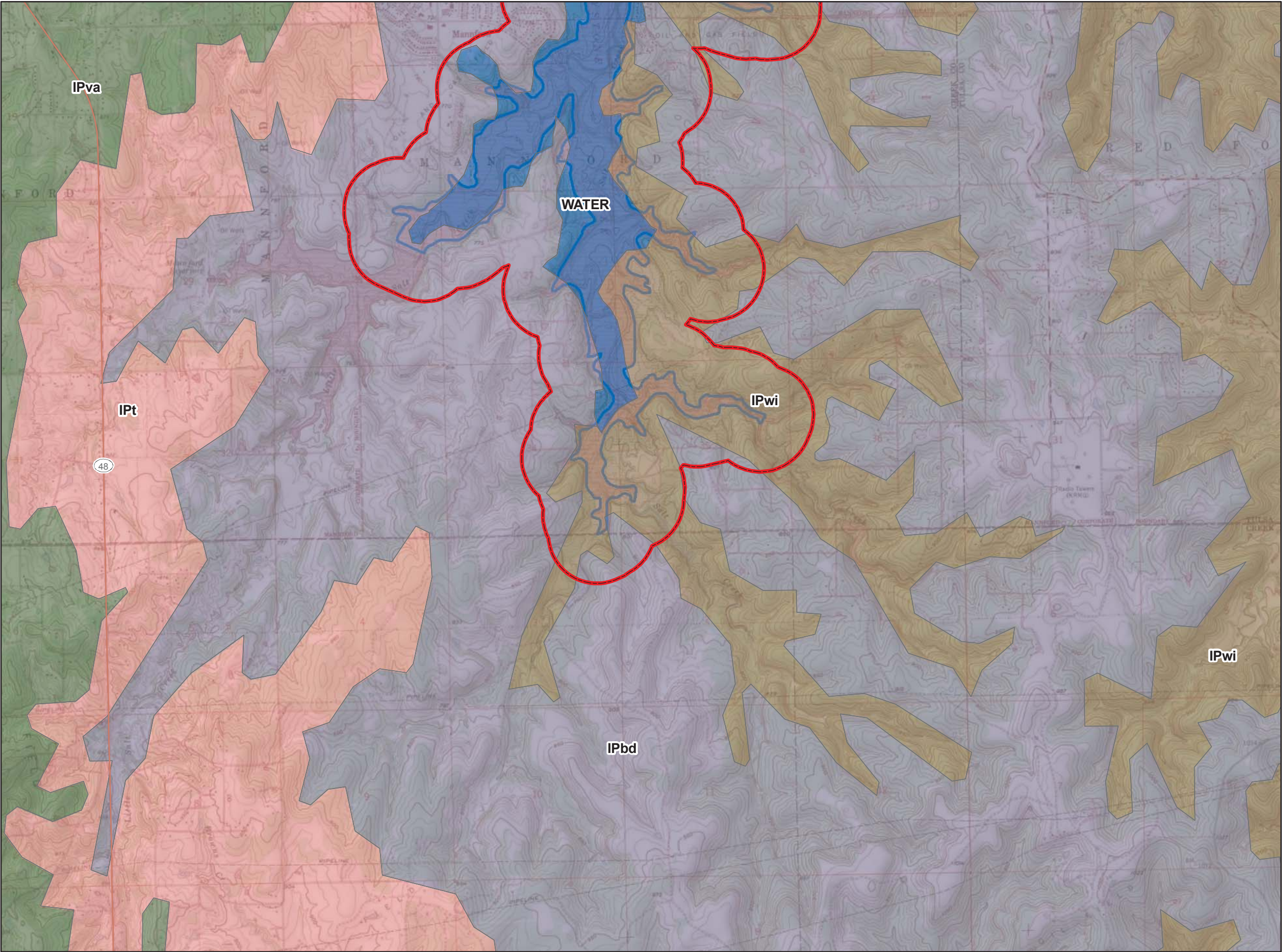


- LEGEND
- Existing Dam
  - Proposed Dam
  - Arkansas River Buffer
  - Interstates
  - US & State Highways
  - Secondary State & County Roads
  - Railroads
  - Keystone Lake

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Qal	Alluvium
Qt	Terrace Deposits
IPva	Vamoosa Formation
IPt	Tallant Formation / (Thurman Sandstone)
IPbd	Barnsdall Formation
IPwi	Wann and Iola Formations
IPa	Atoka Formation
IPch	Chanute Formation
IPnh	Nellie Bly Formation and Hogshooter Limestone
IPd	Dewey Formation
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IPsl	Seminole Formation
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IPhd	Holdenville Shale
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IPbg	Boggy Formation
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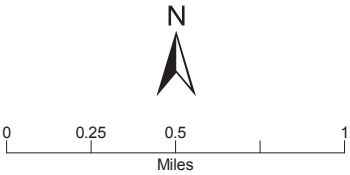




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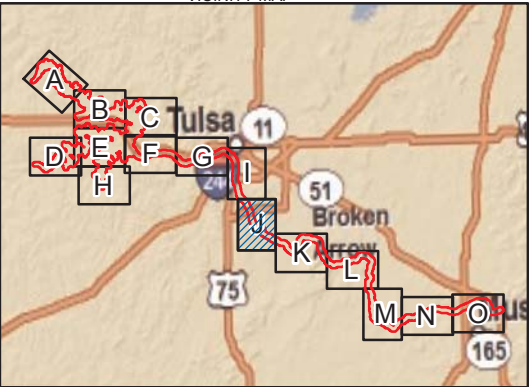
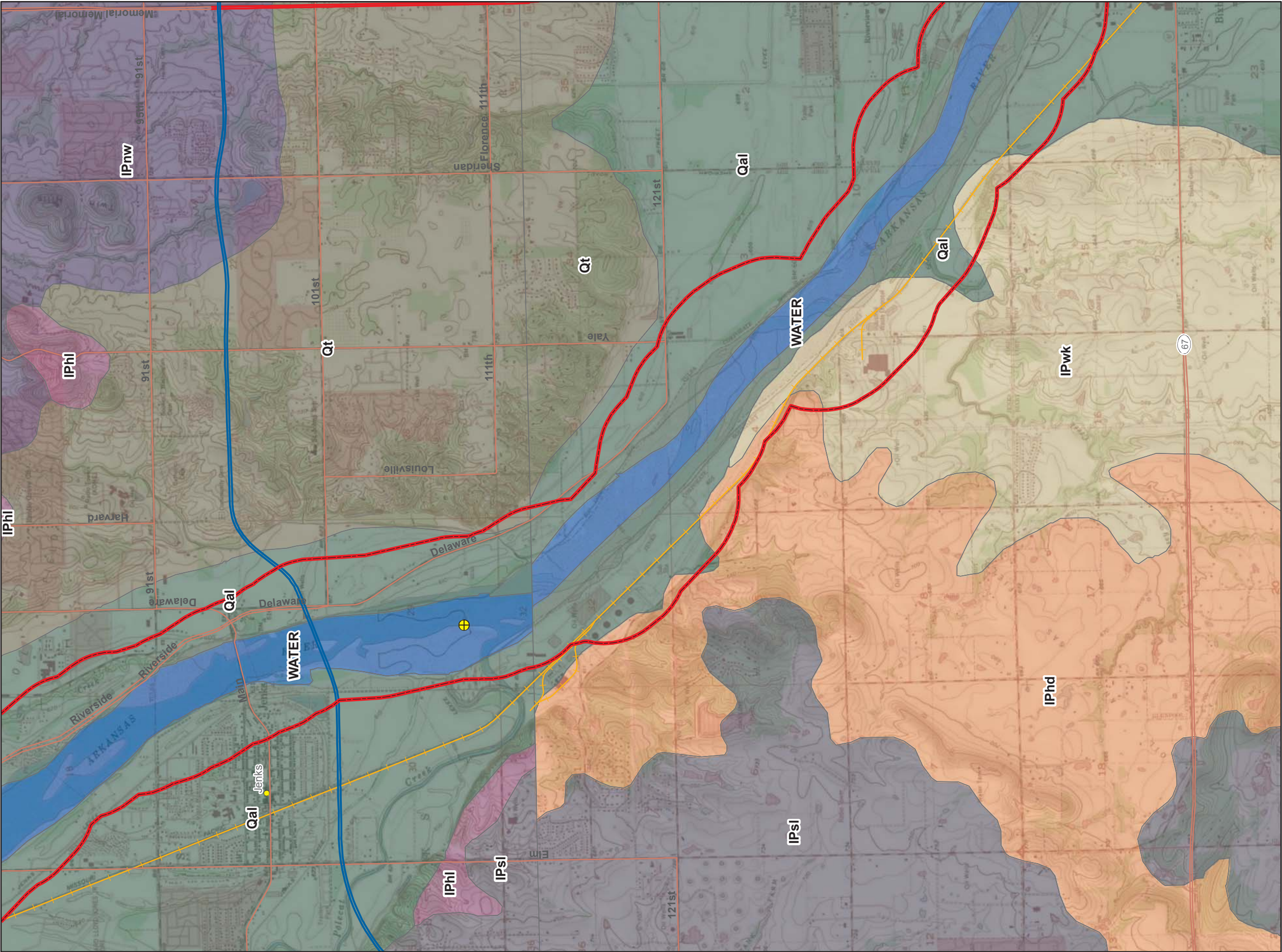
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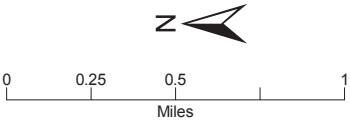




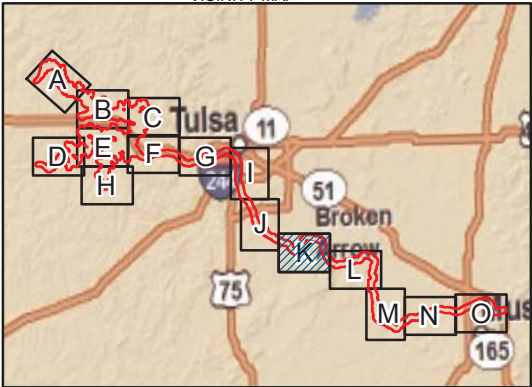
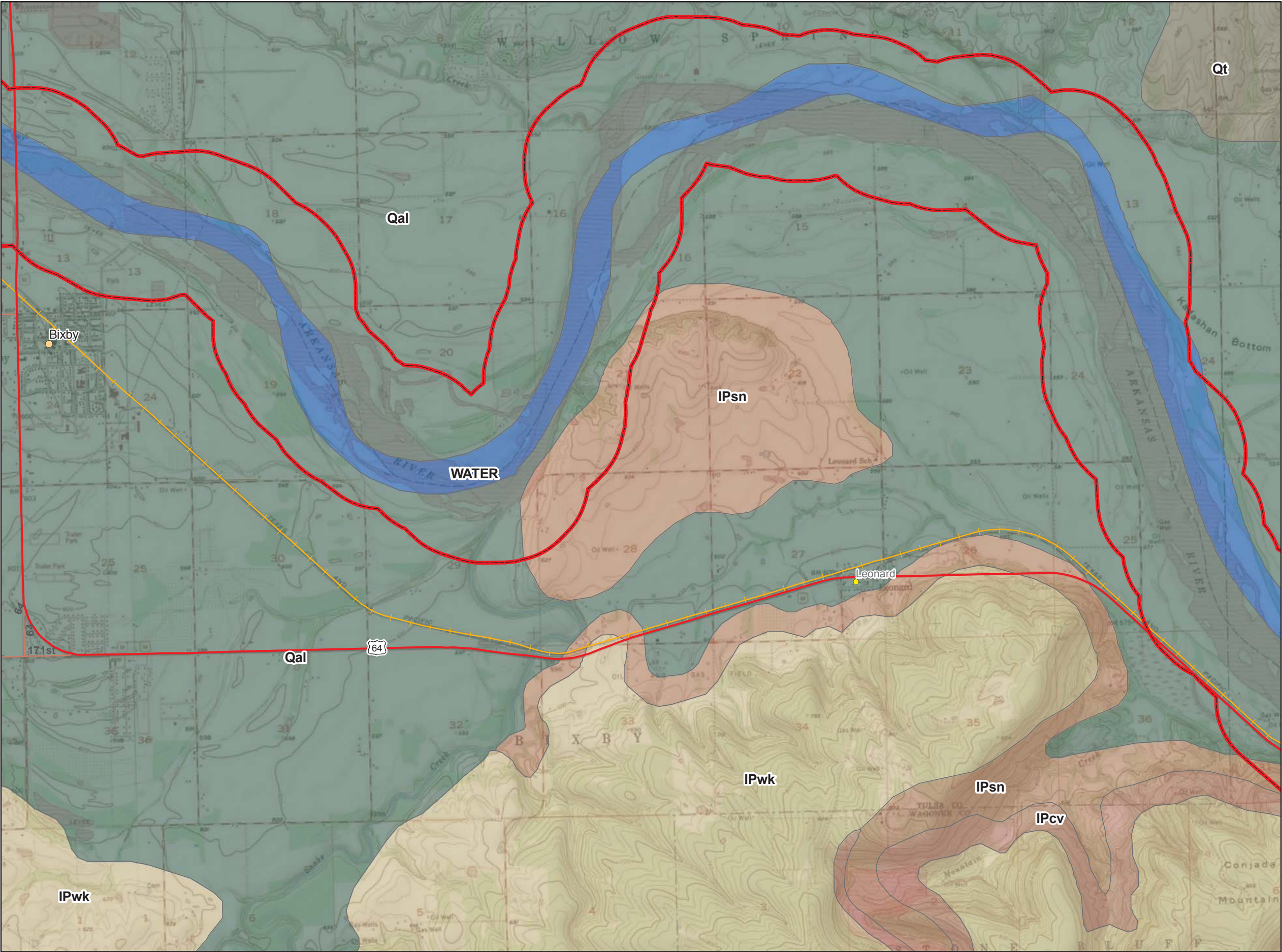
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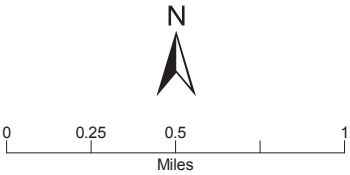




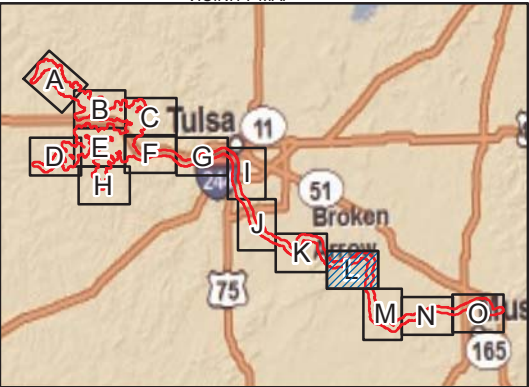
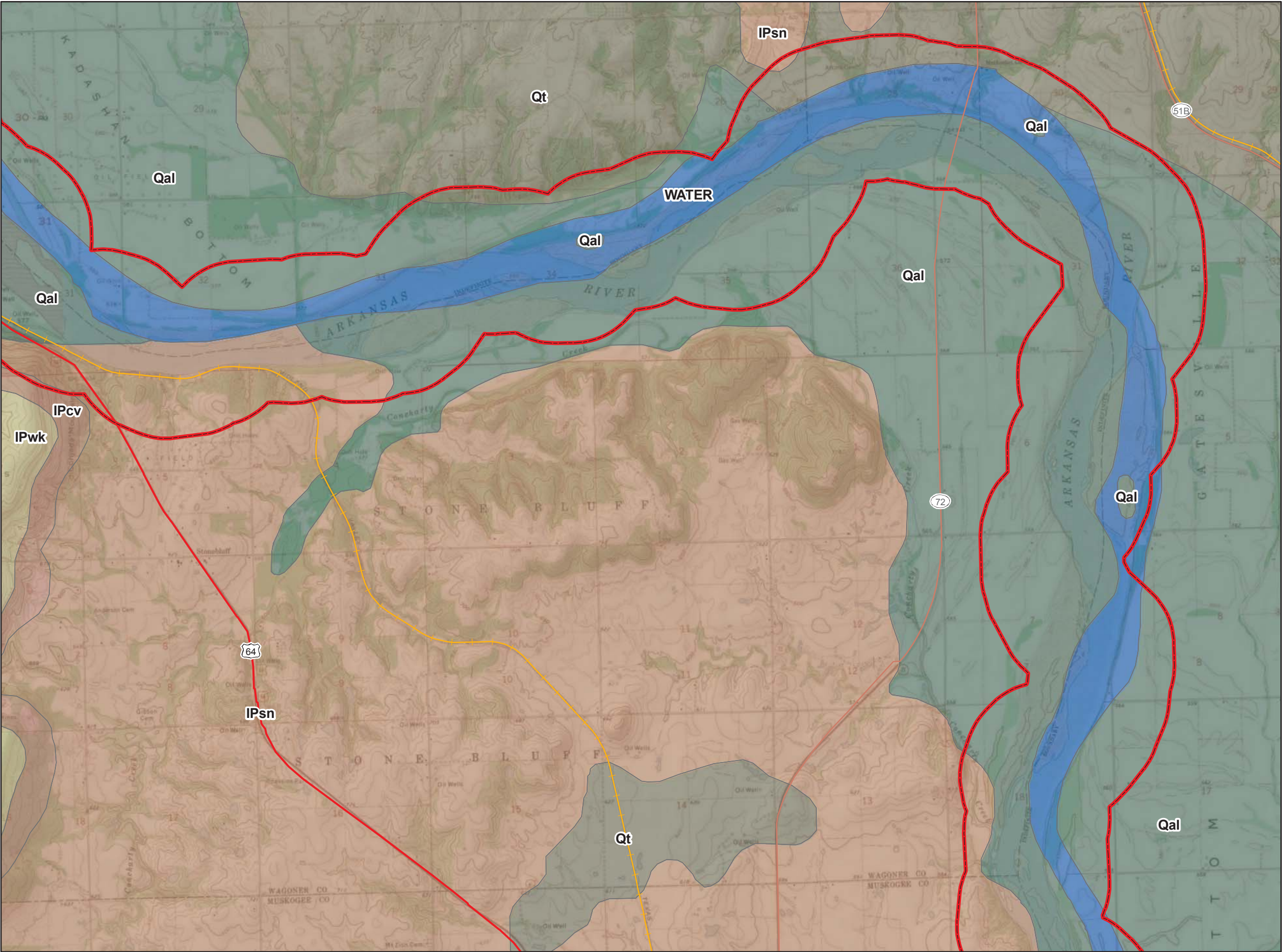
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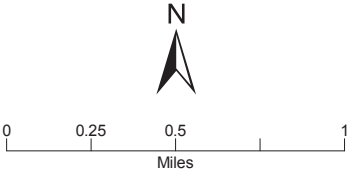




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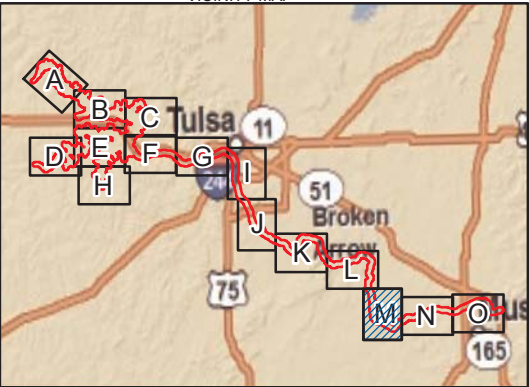
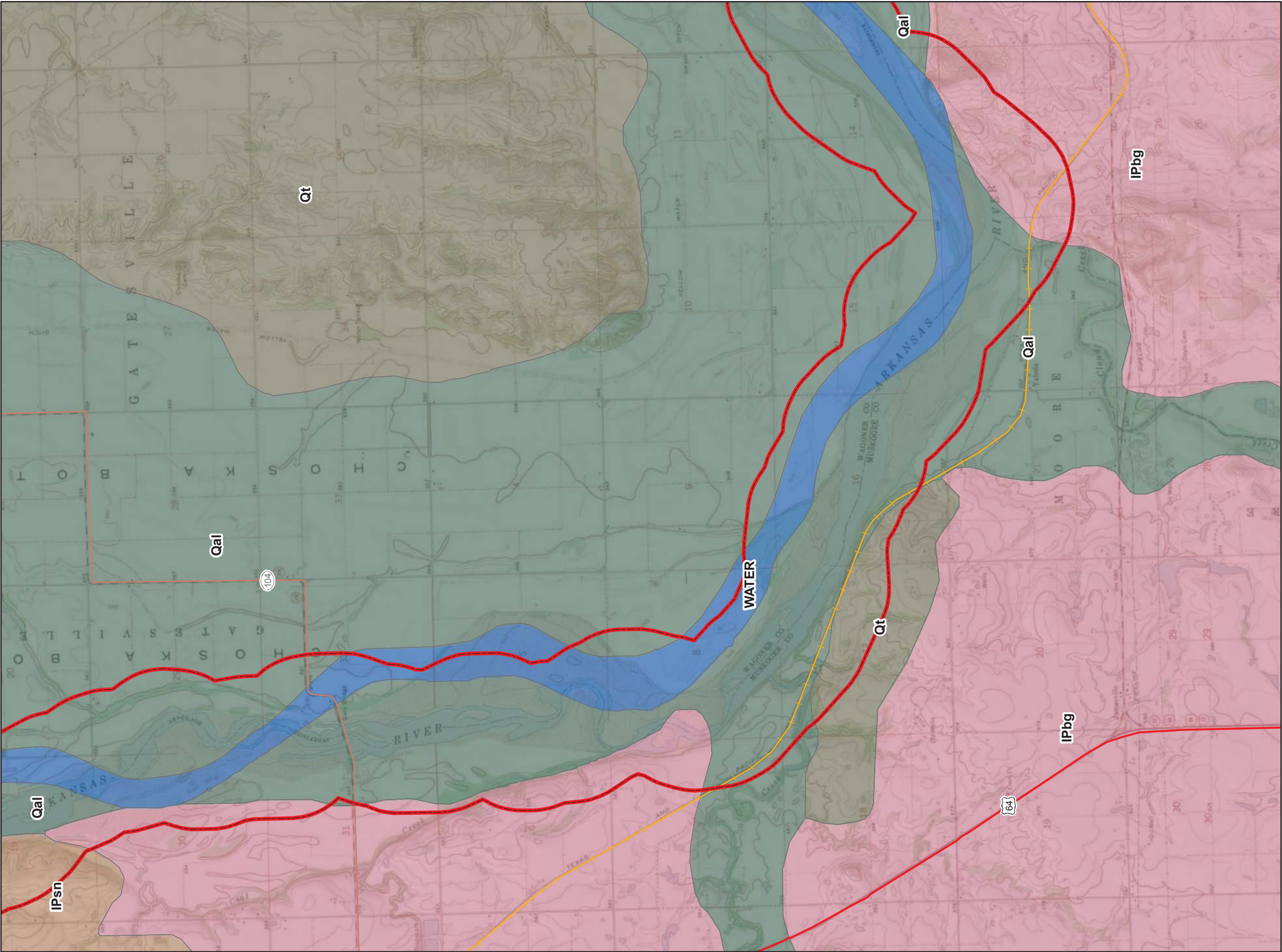
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**Surficial Geology**  
Existing Geotechnical Resource Review  
Arkansas River Corridor  
Tulsa, OK

**Map L**

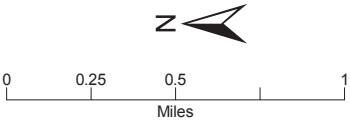




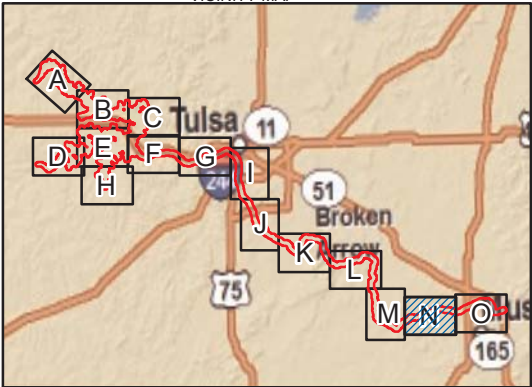
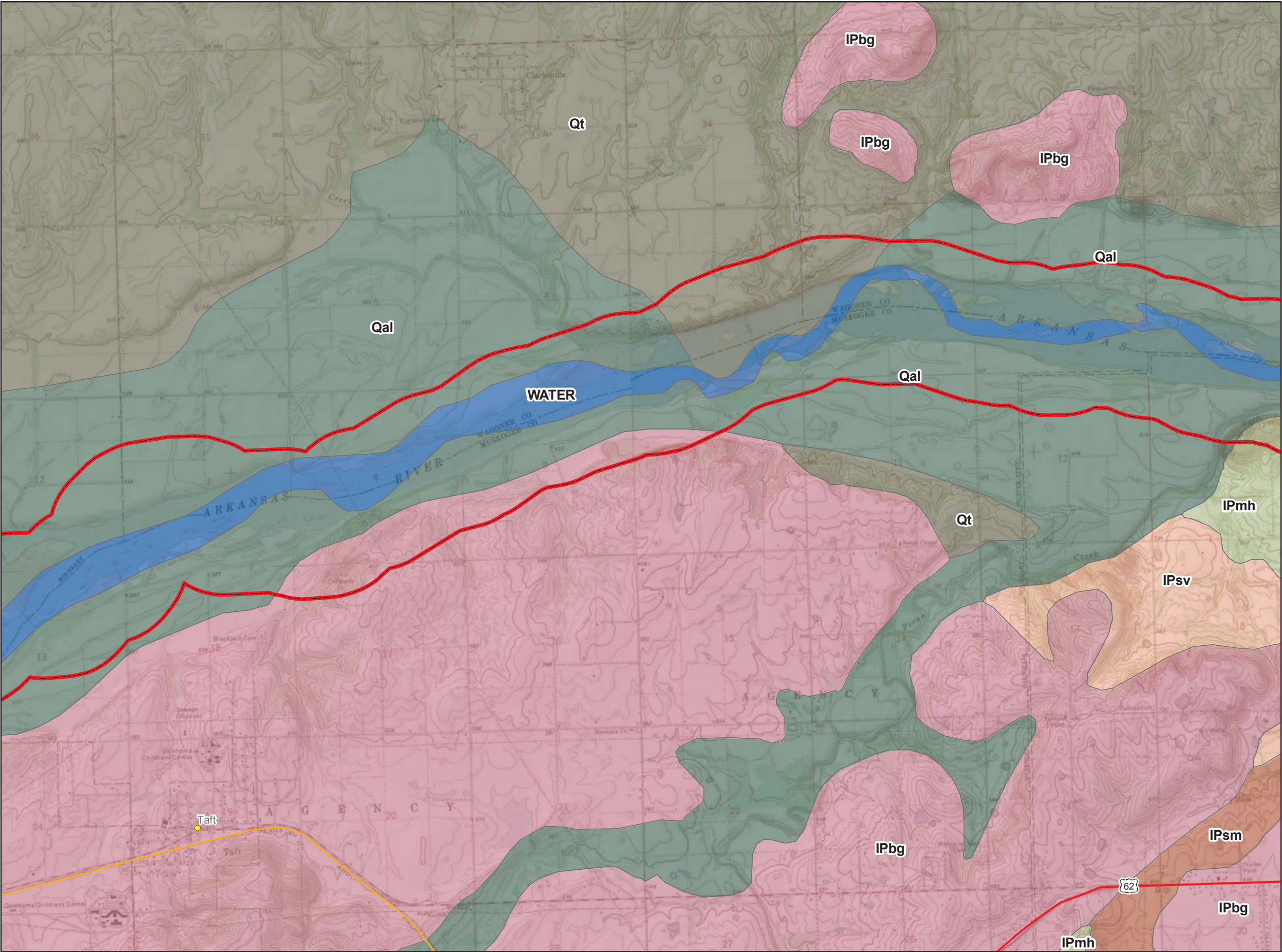
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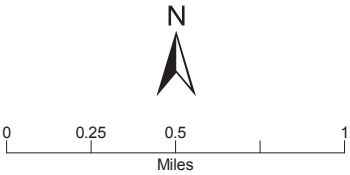




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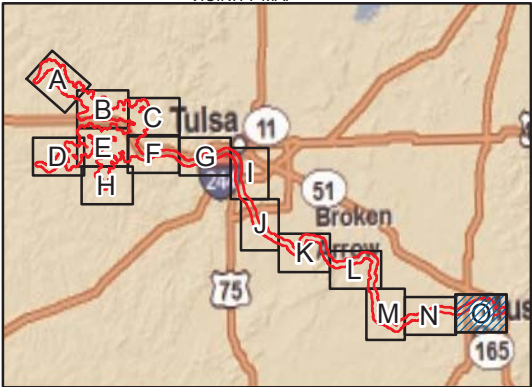
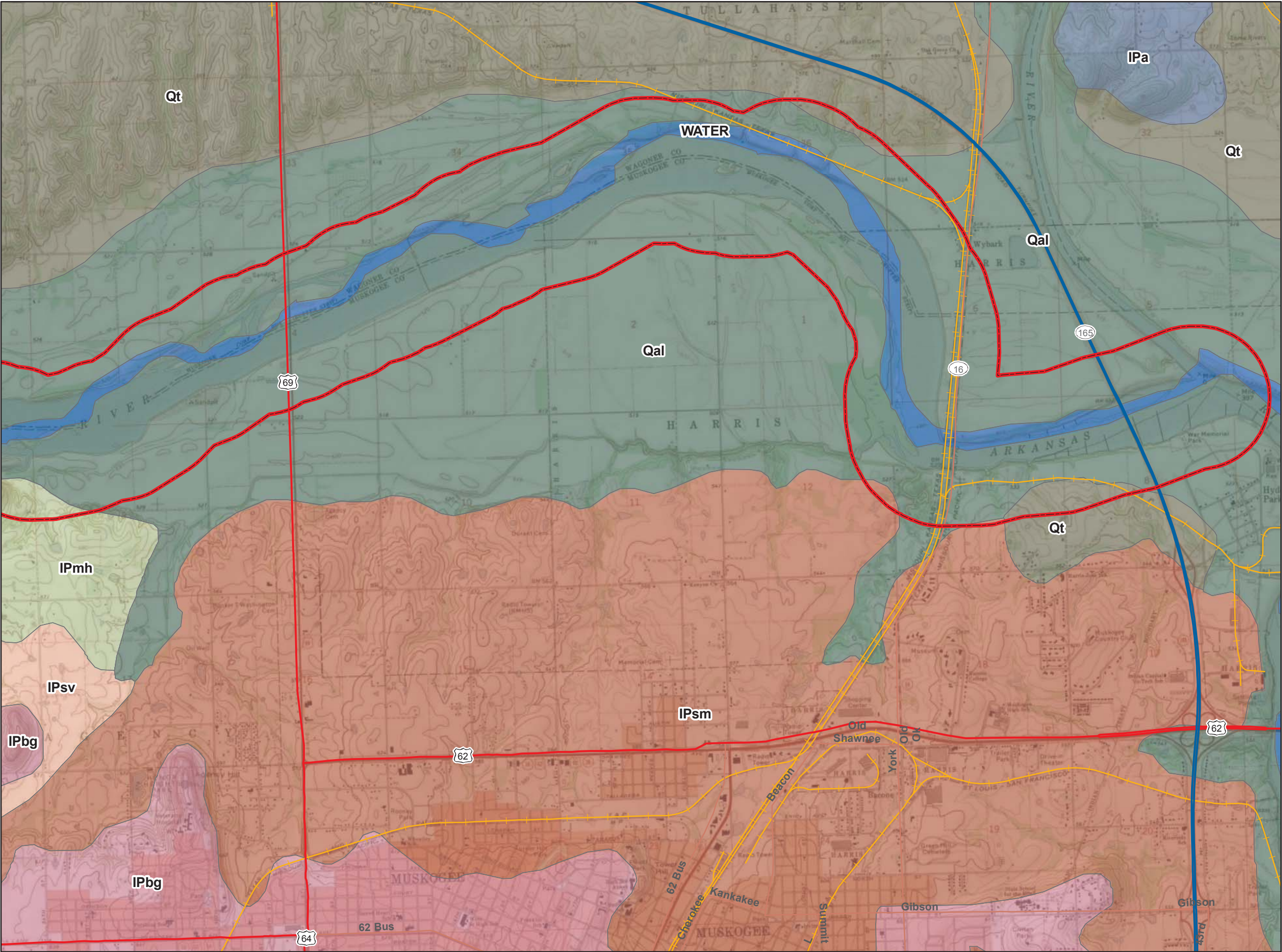
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**Surficial Geology**  
Existing Geotechnical Resource Review  
Arkansas River Corridor  
Tulsa, OK

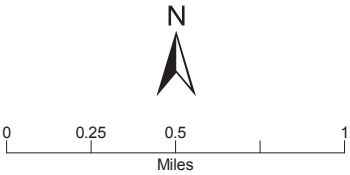
**Map N**





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**Surficial Geology**  
Existing Geotechnical Resource Review  
Arkansas River Corridor  
Tulsa, OK

**Map O**

Attachment B

Approximate Locations of Borings Advanced by Stantec









Attachment C  
Locations of 2002 Borings Advanced by Oklahoma Department  
of Transportation near Bixby

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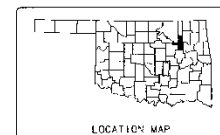
SEE SURVEY DATA SHEETS

STATE OF OKLAHOMA  
DEPARTMENT OF TRANSPORTATION

PLAN OF PROPOSED  
STATE HIGHWAY  
FEDERAL AID PROJECT NO. STPY-172A(008), BRFY-172A(098)  
GRADING, DRAINAGE, SURFACING & BRIDGE PLANS  
U.S. HIGHWAY NO. 64, MEMORIAL DRIVE

## TULSA COUNTY

CONTROL SECTION NO. 64-72-06  
STATE JOB NO. 11186(04)(08)  
CONTRACT NO. 1



INDEX OF SHEETS  
SEE SHEET NO. 2 FOR  
INDEX OF SHEETS AND  
STANDARD DRAWINGS

DESIGN DATA		
ADT 2002	=	18,125
ADT 2022	=	30,800
DHV (TWO-WAY)	=	2,350
K (K <sub>TH</sub> /ADT)	=	10%
D	=	60%
T (%DHV)	=	4%
T (%ADT)	=	6%
T <sub>3</sub> (%ADT)	=	3%
V	=	70 kmph

SCALES 

PLAN 1:500  
PROFILE HOR. 1:500  
VER. 1:50  
LAYOUT MAP 1:25000

STA. 4+633.600 END FEDERAL AID  
PROJECT NO. STPY-172A(008)

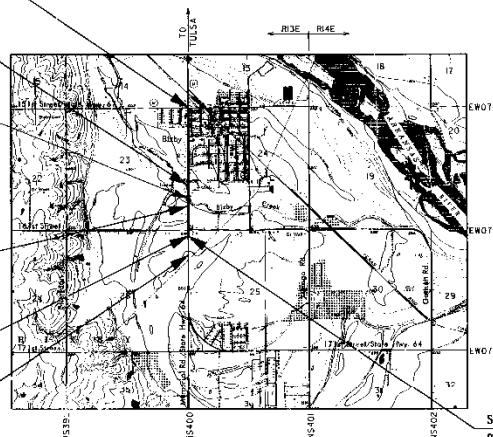
STA. 3+640.000 END FEDERAL AID  
PROJECT NO. BRFY-172A(098)  
BEGIN FEDERAL AID PROJECT  
NO. STPY-172A(008)

Bridge "A" END BRIDGE STA. 3+445.000  
BRIDGE LENGTH 32.000 m  
BEGIN BRIDGE STA. 3+413.000  
LOCATION NO. 7206-0625X  
EXISTING NB NO. 00572  
NEW NB NO. 26791

STA. 3+280.000 END FEDERAL AID  
PROJECT NO. STPY-172A(008)  
BEGIN FEDERAL AID PROJECT  
NO. BRFY-172A(098)

STA. 2+920.000  
END INCIDENTAL CONSTRUCTION

STA. 2+740.00  
BEGIN INCIDENTAL CONSTRUCTION



STA. 2+920.000 BEGIN FEDERAL AID  
PROJECT NO. STPY-172A(008)

J.P. NO. 213,040  
STPY-172,032

ROADWAY LENGTH	353.600 m	1.353 km
BRIDGE LENGTH	0.000 m	0.000 km
PROJECT LENGTH		1.353 km
INCIDENTAL CONSTRUCTION	180.000 m	

EQUATIONS 1-50%

EXCEPTION: 10295  
J.P. NO. 2186(02)  
BREV-172A(0235)

ROADWAY LENGTH	326.400 m	0.326 km
BRIDGE LENGTH	22.000 m	0.032 km
PROJECT LENGTH		0.360 km

2.2. 100% NDNE

LENGTH BASED ON CR

THE ENTIRE PROJECT IS WITHIN THE  
CORPORATE LIMITS OF THE CITY OF BIXBY

## GROUP 2014: ITN 5

[illegible]

### AFTTC PLANS

Prepared by:  
**HJB-Garver**  
Engineers  
5411 South 12th East Ave.  
Tulsa, Oklahoma

1990 AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY  
CONSTRUCTION - METRIC GOVERN, APPROVED BY THE  
U.S. DEPARTMENT OF TRANSPORTATION, FEDERAL HIGHWAY  
ADMINISTRATION, SEPTEMBER 21, 1989.



LXLAHMA  
DEPARTMENT OF TRANSPORTATION

DATE APPROVED \_\_\_\_\_

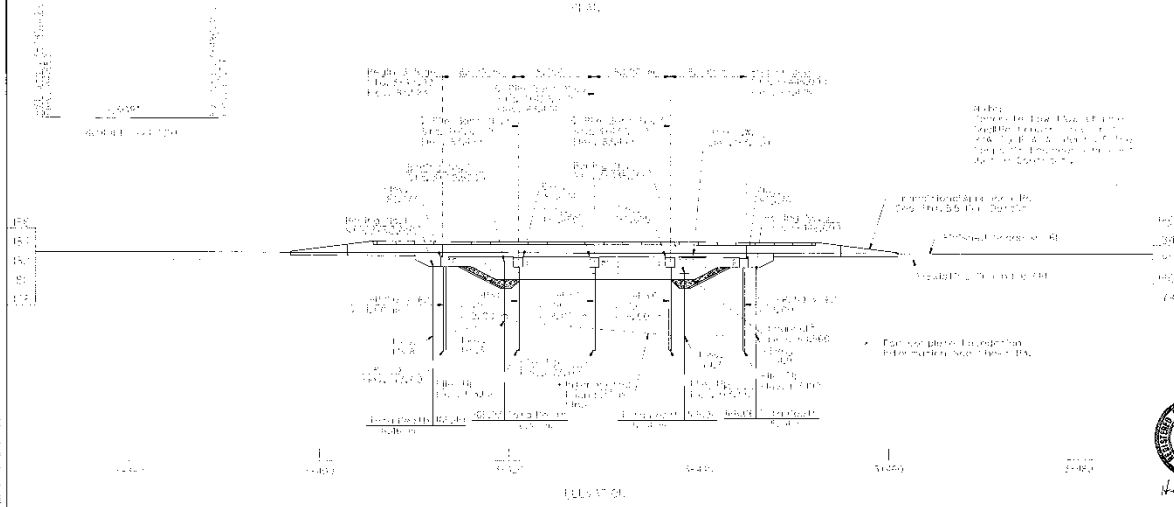
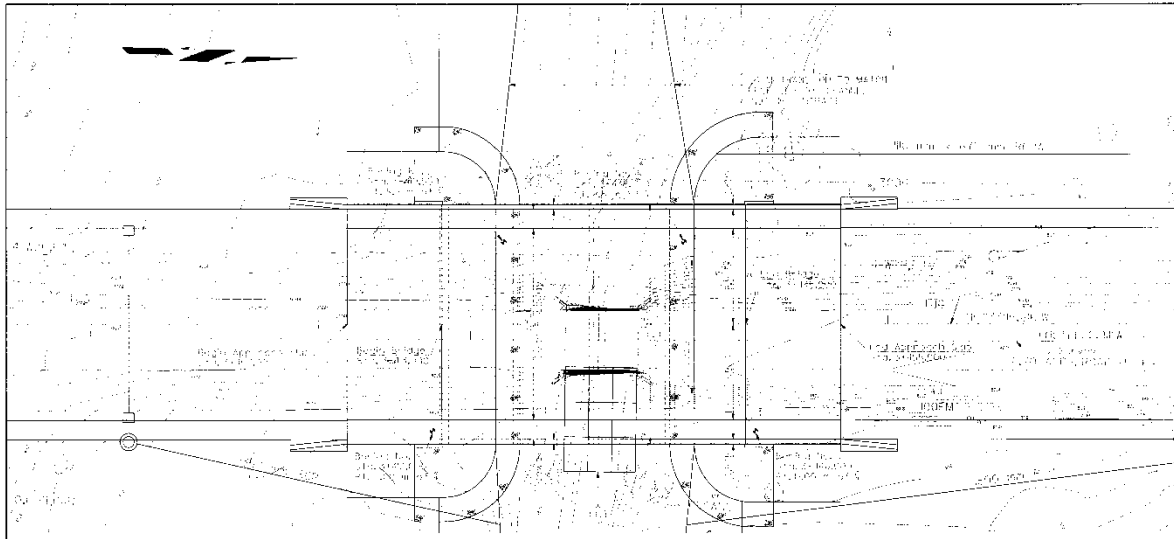
BY	CHIEF ENGINEER
C/O 1010111 S A BDO E T A D ST	

DEPARTMENT OF TRANSPORTATION  
FEDERAL HIGHWAY ADMINISTRATION

DATE APPROVED \_\_\_\_\_

BY                      DIVISION ADMINISTRATOR

CLACK COUNTY



**NOTES:**

1. The bridge is to be constructed of reinforced concrete.
2. The bridge deck is to be 20 feet wide.
3. The bridge approach roads are to be 10 feet wide.
4. The bridge piers are to be 10 feet wide.
5. The bridge abutments are to be 10 feet wide.
6. The bridge foundations are to be 10 feet wide.
7. The bridge structure is to be 10 feet wide.
8. The bridge structure is to be 10 feet wide.
9. The bridge structure is to be 10 feet wide.
10. The bridge structure is to be 10 feet wide.

**REVISIONS:**

NO.	DESCRIPTION	DATE
1	Original design	10/1/50
2	Revised design	10/1/50
3	Revised design	10/1/50
4	Revised design	10/1/50
5	Revised design	10/1/50
6	Revised design	10/1/50
7	Revised design	10/1/50
8	Revised design	10/1/50
9	Revised design	10/1/50
10	Revised design	10/1/50

**STATE OF OKLAHOMA**

**HERBERT J. POWERS, JR.**  
1950  
OKLAHOMA

*Herbert J. Powers, Jr.*  
State

Attachment D  
Locations of 1994 Borings Advanced by Oklahoma Department  
of Transportation at Bridge Piers near Jenks

---



SUB-6.1.94

## SURVEY DATA

1. HORIZONTAL CONTROL:  
 A. HORIZONTAL CONTROL FOR THIS SURVEY IS THE USC & GS OKLAHOMA STATE PLANE COORDINATE SYSTEM, LAMBERT PROJECT, NORTH ZONE. ALL COORDINATES SHOWN ARE OKLAHOMA STATE PLANE COORDINATES.  
 B. ACCURACY - 3RD ORDER OR BETTER.

2. BEARINGS:  
 THE BEARINGS SHOWN HEREIN ARE GRID BEARINGS DERIVED FROM THE USC & GS OKLAHOMA PLANE COORDINATE SYSTEM AND ARE NOT ASTRONOMICAL.

3. VERTICAL CONTROLS:  
 A. LEVEL DATUM IS MEAN SEA LEVEL (USC&GS).  
 B. ACCURACY - 3RD ORDER OR BETTER.

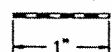
## TRAFFIC DATA \*

ADT- 1991 = 16,000  
 ADT- 2011 = 26,000  
 DHV = 2,600  
 D = 55%  
 T(XADT) = 6%  
 V = 45 MPH

\* NOTE: ADDITIONAL TRAFFIC DATA IS SHOWN ON SIGNALIZATION SHEETS.

## SCALES

PLAN 1" = 20'  
 PROFILE HOR. 1" = 20'  
 VER. 1" = 5'  
 LAYOUT MAP 1" = 800'



## CONVENTIONAL SYMBOLS

- PROPOSED ROAD  
 RAILROADS  
 RANGE & TOWNSHIP  
 SECTION LINES  
 QUARTER SECTION LINES  
 FENCES  
 GROUND LINE  
 EXISTING ROADS  
 BASE LINE (SBL & CRL)  
 GRADE LINE  
 TELEPHONE & TELEGRAPH  
 POWER LINES  
 OIL WELLS  
 BUILDINGS  
 DRAINAGE STRUCTURE - IN PLACE  
 DRAINAGE STRUCTURE - NEW  
 RIGHT-OF-WAY LINES - EXISTING  
 RIGHT-OF-WAY LINES - NEW  
 TREE  
 TRAFFIC SIGNAL CASE  
 TRANSFORMER  
 SIGN  
 FIRE HYDRANT  
 TELEPHONE TERMINAL  
 MANHOLE - TELEPHONE  
 MANHOLE - SANITARY SEWER  
 MANHOLE - STORM SEWER  
 GAS METER  
 WATER METER  
 LIGHT POLE  
 UTILITY POLE  
 TELEPHONE UNDERGROUND  
 WATER  
 GAS  
 SANITARY SEWER  
 STORM SEWER

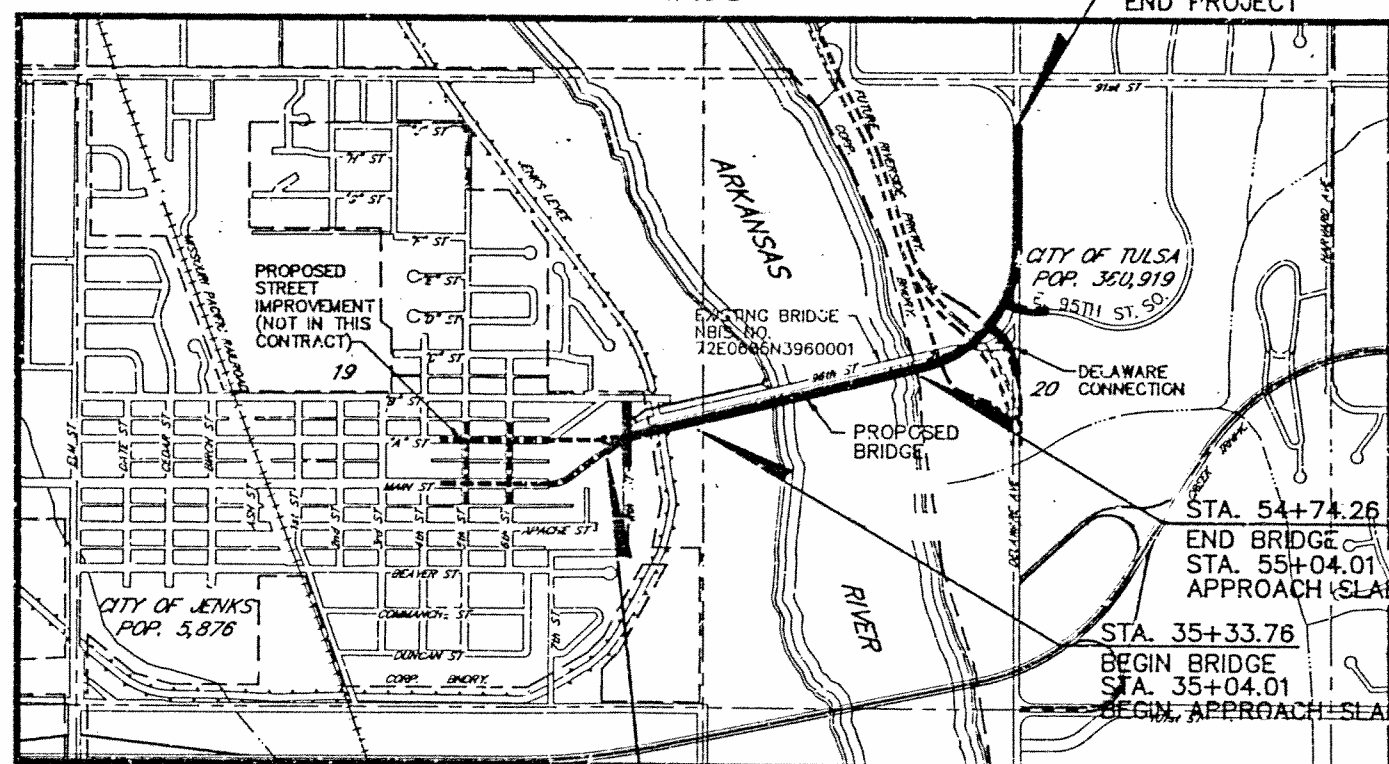
THE 1988 OKLAHOMA STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION GOVERN, APPROVED BY THE U.S. DEPARTMENT OF TRANSPORTATION, FEDERAL HIGHWAY ADMINISTRATION, JANUARY 3, 1989. SUPPLEMENTAL SPECIFICATIONS TO THE 1988 STANDARD SPECIFICATIONS GOVERN OVER THE STANDARD SPECIFICATIONS.

STATE OF OKLAHOMA  
 DEPARTMENT OF TRANSPORTATION

PLAN OF PROPOSED  
 CITY BRIDGE

FEDERAL AID PROJECT NO. MA-M-STP-072B (410) IG  
 ARKANSAS RIVER BRIDGE AND WEST APPROACH: CITY OF JENKS  
 EAST APPROACH: CITY OF TULSA  
 TULSA COUNTY

STATE JOB NO. 07313 (04)  
 LOCATION NO. 72E0685N3960001  
 NBIS NO. 10573  
 R 13 E



## LAYOUT MAP

ROADWAY LENGTH 3143.00 0.595 MI.  
 BRIDGE LENGTH 1940.50 FT. 0.367 MI.  
 PROJECT LENGTH 0.962 MI.

EQUATIONS: ON WEST APPROACH:  
 STA. 32+04.93 BACK-  
 STA. 35+04.01 FWD.  
 ON EAST APPROACH:  
 STA. 55+04.01-  
 STA. 9+86.82 96th ST.

MICROFILMED 7.14.94  
 SCALE 2 inches

DELETED SHEET 4C 7-14-94

FED. ROAD DIST. NO.	STATE	F.A. PROJ. NO.	FISCAL YEAR	SHT. NO.	TOTAL SHEETS
5	OKLA		1994	1	11

## INDEX OF SHEETS

REV.	PROJECT	NUMBER	DATE
1		6-15-94	
2-4, 4A			
4B			
5, 5A			
5B, 5C			
6			
7			
8-9, 9A			
10-13, 13A			
14-15			
16-20, 20A, 21-25, 25A			
26			
27-30			
31			
32-35			
36-37			
38-42			
43			
44-45			
46, 46A, 47, 48, 48A, 48B, 48C			
48D, 48E, 48F, 48G			
48H			
49-52			
53			
54-56			
57-61			
62			
63-68			
69-70			
70A			
71, 71A			
72			
73-75			
76-99			

## ROADWAY STANDARDS:

SPI-2-3	WCR-1-3	RCB-1B-4
PCES-2-2	MI-1-2	PUD 1-23
MFC-2-4	MD-2-31	SUEL-4-0
MJB-1-3	FHTCP-1-1	
SSCD-1-15	ASCD-3-3	
CICI-1-11	CSCD-3-5	
SBI-2-3	CIG-1-2	
SSIF-2-4	LECS-2-4	

## BRIDGE STANDARDS:

P-1-16	SEJ-3-3
	SFP-1-2

## SIGNING STANDARDS:

GMS-1-22	ICD-15-0	PM-7-5	CWS-2-8
FGS-1-34	ICD-15-1	ICD-1-11	ICD-1A-1
SPA-1-17	ICD-3-11	ICD-2-8	
SPA-2-5	ICD-4-8	ICD-3-5	
SSA-1-1	ICD-1-21	ICD-5-9	
PM-1-16	ICD-3-7	ICD-8-7	
PM-3-10	ICD-4-6	ICD-10-5	
PM-4-12	ICD-5-10	CWS-1-7	
PM-5-4	ICD-1-10	CWS-3-8	
RSO-1-25	SBS-2-9	CWS-4-7	
RSO-2-6	SBS-3-9	CWS-8-6	
ICD-9-4	SBS-4-7	PM-8-3	

## LIGHTING STANDARDS:

CCD-1-4	GMF-1-6	HLP-2-2	HLD-1-2
CCD-2-2	BBD-1-3	HLP-3-1	SPD-1-10
PBD-1-8	HLP-1-5	PPD-1-3	SCD-1-5
			TWO-1-3

APPROVED BY:

CHARLES HARDT, DIRECTOR OF PUBLIC WORKS, CITY OF TULSA

BY:

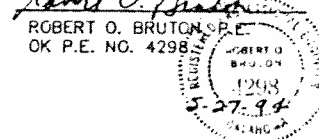
HAROLD G. WILMOTT, MAYOR, CITY OF JENKS

ENGINEERS: WEST ROADWAY & BRIDGE  
FHC, Inc.EAST SIDE ROADWAY  
BRUTON KNOWLES & LOVE, Inc.

BY:

JON D. NELSON, P.E.  
OK P.E. NO. 11442ROBERT O. BRUTON, P.E.  
OK P.E. NO. 4298

BY:

D. NEWELL PULLIAM, JR., P.E.  
OK P.E. NO. 15688OKLAHOMA  
DEPARTMENT OF TRANSPORTATIONDEPARTMENT OF TRANSPORTATION  
FEDERAL HIGHWAY ADMINISTRATION

DATE APPROVED

DATE APPROVED

BY: CHIEF ENGINEER

BY: DIVISION ADMINISTRATOR

F. A. Project No. MA-M-STP-072B(410)IG Sheet No. 1

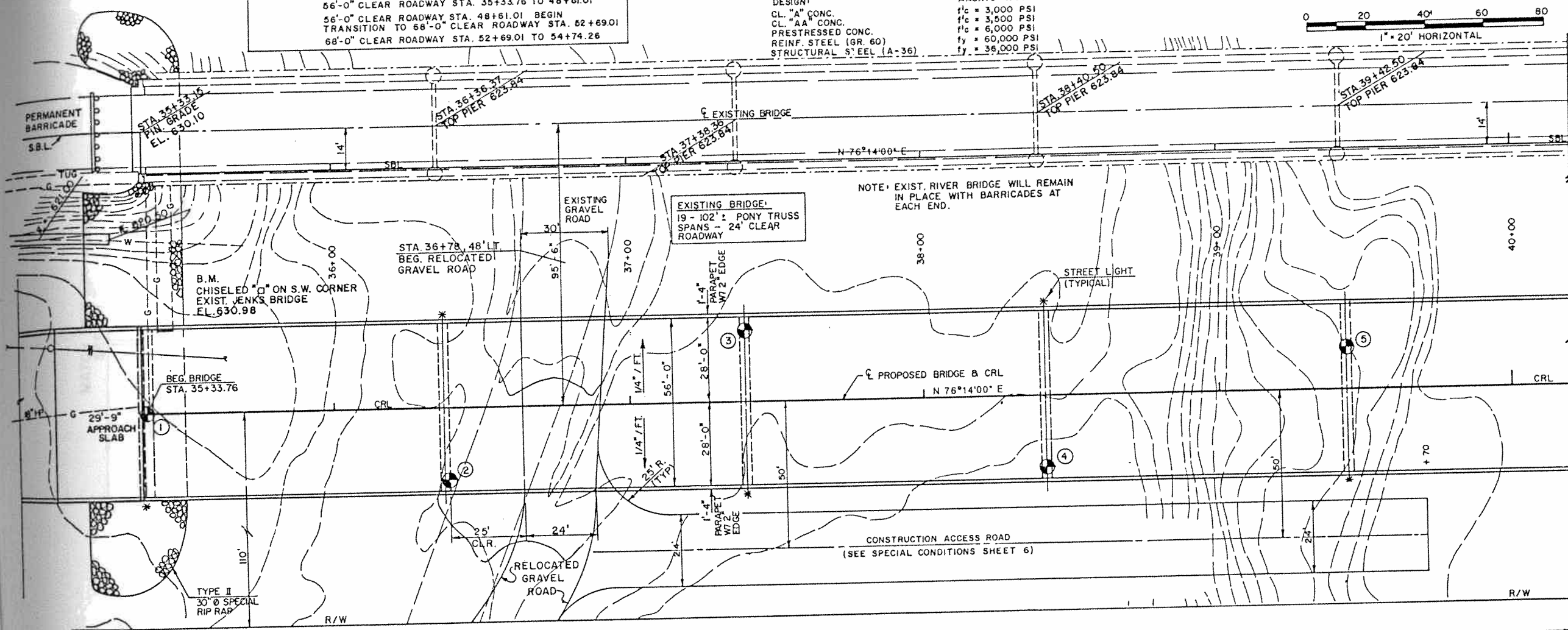
BRIDGE "A" 19-102'-0" PRESTRESSED CONC. BEAM SPANS WITH 1'-4" CONC. PARAPETS.  
 56'-0" CLEAR ROADWAY STA. 35+33.76 TO 48+61.01  
 56'-0" CLEAR ROADWAY STA. 48+61.01 BEGIN  
 TRANSITION TO 68'-0" CLEAR ROADWAY STA. 62+69.01  
 68'-0" CLEAR ROADWAY STA. 52+69.01 TO 54+74.26

DESIGN DATA  
 LOADING: HS 20 W/ 20 psf (FUTURE WEARING SURFACE & OKLA. OVRD. TRUCK)  
 LOAD FACTOR DESIGN  
 DESIGN: AASHTO-1989  
 CL. "A" CONC.  $f'_c = 3,000$  PSI  
 CL. "AA" CONC.  $f'_c = 3,500$  PSI  
 PRESTRESSED CONC.  $f'_c = 6,000$  PSI  
 REINF. STEEL (GR. 60)  $f_y = 60,000$  PSI  
 STRUCTURAL STEEL (A-36)  $f_y = 36,000$  PSI

FED. ROAD DIST. NO.	STATE	J/P NO.	PROJECT NO.	SHEET NO.	TOTAL SHEETS
6	OKLA.	07313(04)	1994	49	111

REVISIONS	DATE
ELEV	6-24-94



BENCH MARK: CHISELED "C" ON N.E. CORNER CONC. SLAB OF GAS REGULATOR, APPROX. 89.5' E. & 70.5' N. OF EAST END EXIST. BRIDGE. ELEV. 630.99

### HYDRAULIC DATA

Q500 = 480,000 cfs  
 V500 = 62 fps  
 H.W. EL. (500 YR.) = 626.20  
 Q100 = 170,000 cfs  
 V100 = 7.22 fps  
 H.W. EL. (100 YR.) = 615.28  
 Q50 = 105,000 cfs  
 V50 = 5.93 fps  
 H.W. EL. (50 YR.) = 610.90

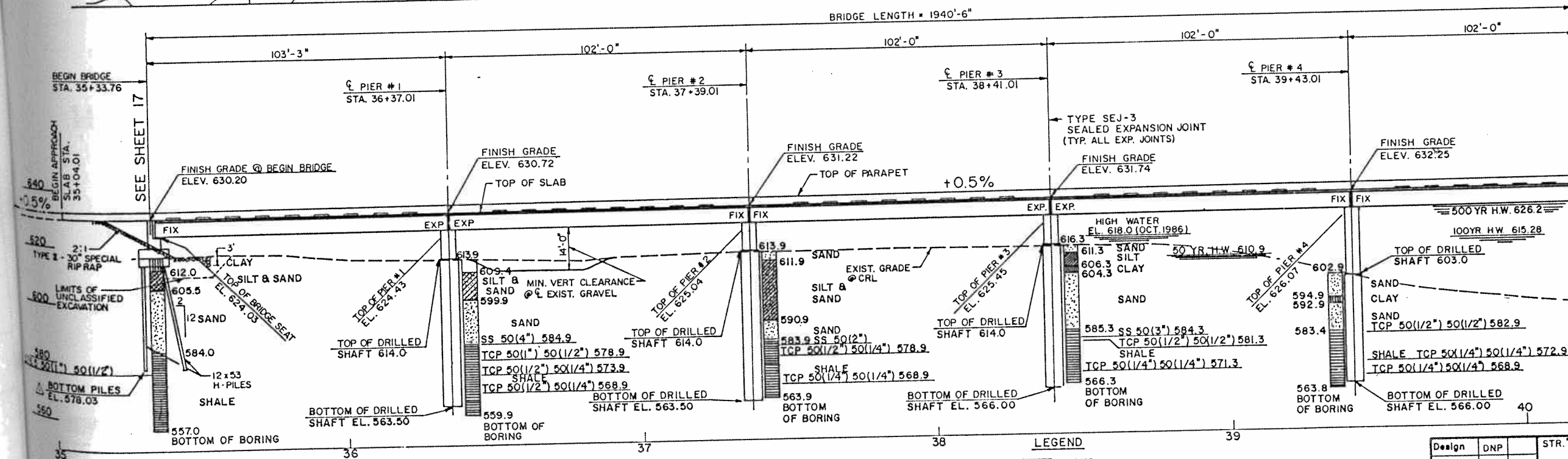
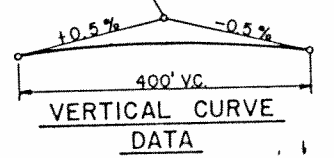
DRAINAGE AREA:  
 TOTAL: 74,615 SQ. MI.  
 UNCONTROLLED: 22,621 SQ. MI.  
 PIER SCOUR DEPTH (CALC.) = 10.4 FT.  
 EXTREME HIGH WATER - N/A

### FOUNDATION LOADS

STEEL PILING: 59.9 TONS  
 DRILLED SHAFT: ALLOWABLE BEARING = 30 tsf  
 ALLOWABLE FRICTION = 2 tsf

LOCATION	DESIGN LOAD PER SHAFT	CAPACITY PER SHAFT	CAPACITY BEARING	TOTAL CAPACITY
PIERS 1-18	532	283	477	760

\* NEGLECT 4' OF ROCK FOR FRICTION  
 PVI STA. 45+04.01  
 PVI EL. 635.05



STEEL PILING:  
 ALL ABUTMENT PILING SHALL BE DRIVEN THROUGH THE COMPACTED FILL. STEEL PILING SHALL BE DRIVEN TO POINT BEARING ON SOLID FOUNDATION MATERIAL AT THE APPROXIMATE ELEVATION SHOWN ON THE PLANS. IF PRACTICAL REFUSAL IS NOT OBTAINED AT THIS ELEVATION, DRIVING SHALL CONTINUE UNTIL THE MINIMUM REQUIRED BEARING OF 120 TONS IS OBTAINED.  
 THE LENGTH OF STEEL PILING SHOWN ON THE PLANS IS FOR ESTIMATING PURPOSES ONLY.

### LEGEND

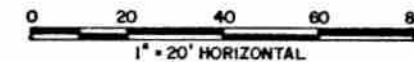
- SAND
- SILT
- CLAY
- SHALE

Design	DNP
Drawn	JMB
Checked	
Approved	
Squad	

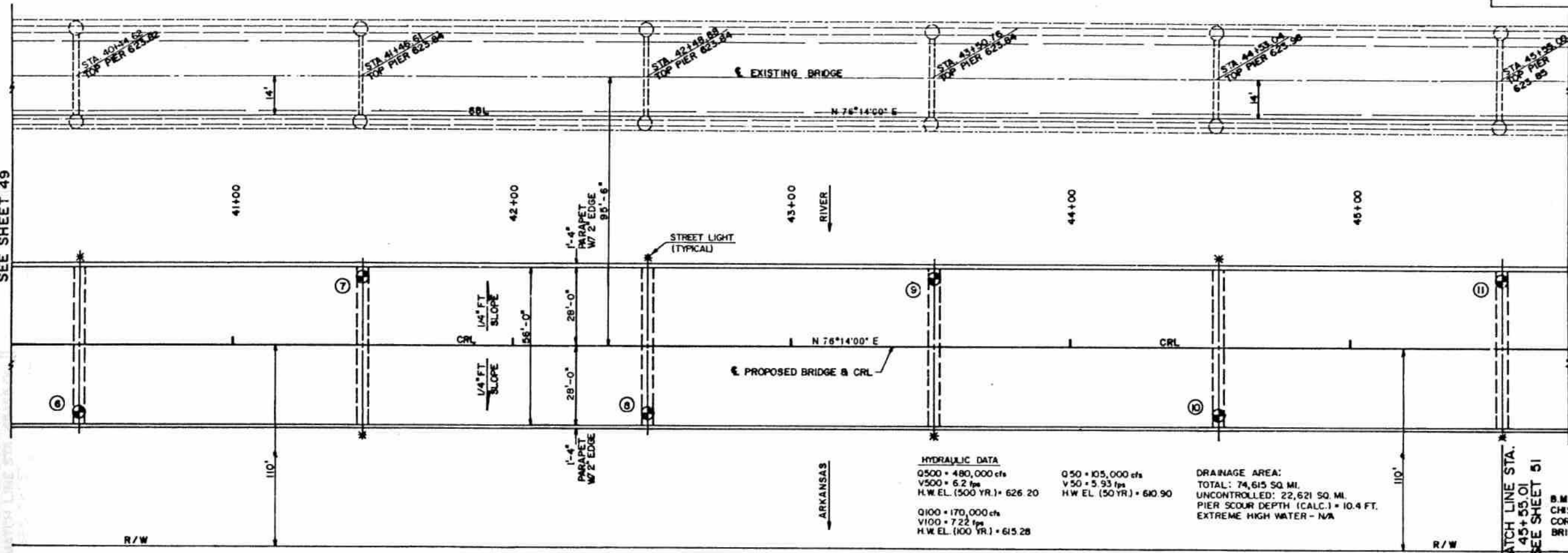
STR. "A" ARKANSAS RIVER  
 GENERAL ELEV. AND PLAN  
 19-102'-0" PRESTRESSED CONC. BEAM SPANS  
 56'-0" CLR. RDY. W/ CONC. PARAPET  
 STATE J/P NO. 07313(04) Sheet No. 49



MATCH LINE STA. 40+20  
SEE SHEET 49



REV.	DATE	BY	CHKD.	APP. NO.	PROJECT	SHEET NO.	TOTAL SHEETS
1	07/23/04	DRG.			19-102-00	50	111

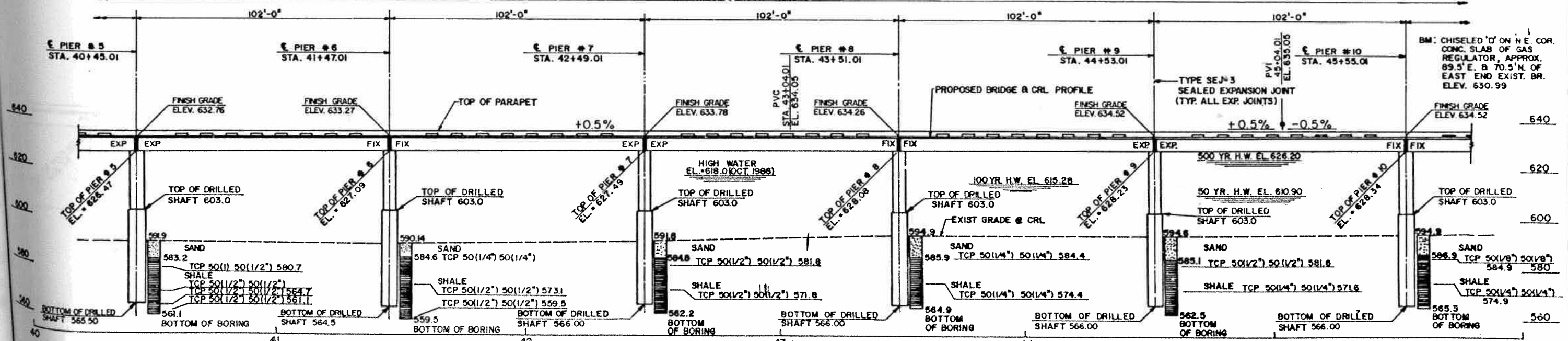


**HYDRAULIC DATA**  
 Q500 = 480,000 cfs  
 V500 = 6.2 fps  
 H.W. EL. (500 YR.) = 626.20  
 Q100 = 170,000 cfs  
 V100 = 7.22 fps  
 H.W. EL. (100 YR.) = 615.28  
 Q50 = 105,000 cfs  
 V50 = 5.93 fps  
 H.W. EL. (50 YR.) = 610.90  
**DRAINAGE AREA:**  
 TOTAL: 74,615 SQ. MI.  
 UNCONTROLLED: 22,621 SQ. MI.  
 PIER SCOUR DEPTH (CALC.) = 10.4 FT.  
 EXTREME HIGH WATER - N/A

B.M. CHISELED "D" ON S.W. CORNER EXIST. JENKS BRIDGE ELEV. 630.98

PLAN

BRIDGE LENGTH = 1940'-6"

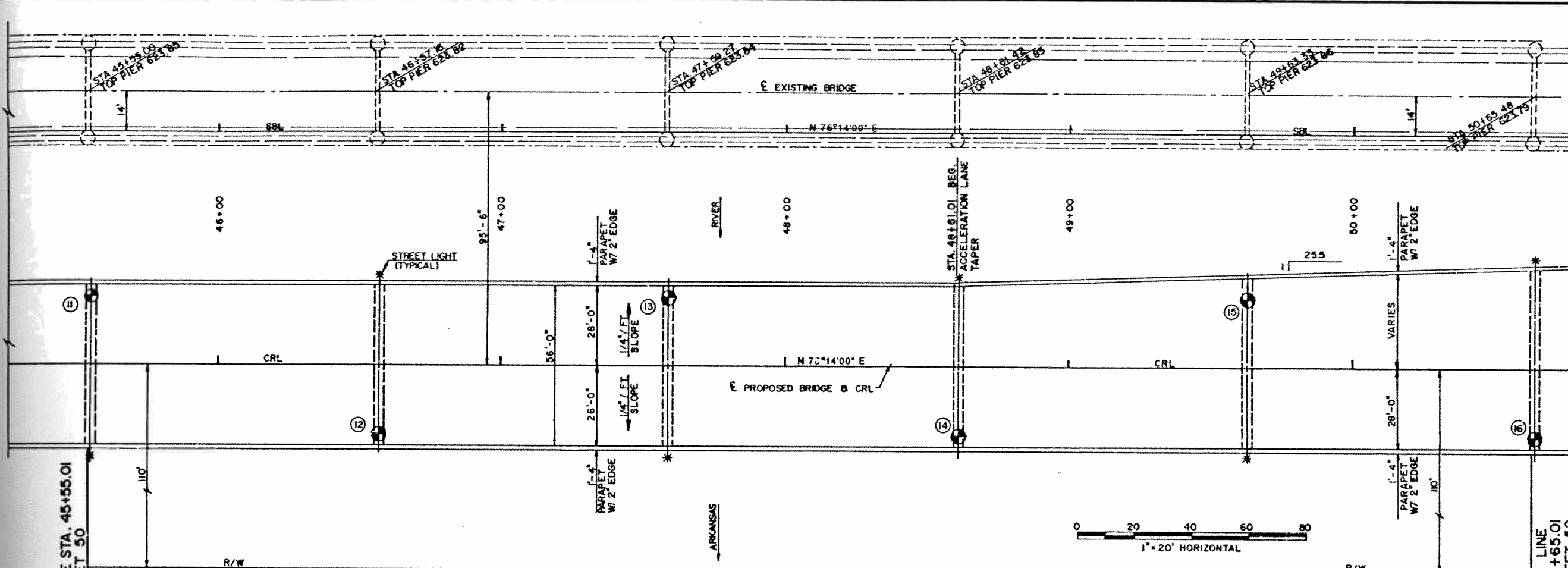


ELEVATION  
SC: 1" = 20'

**LEGEND**  
 SAND  
 SHALE

Design	DNP
Drawn	SGG
Checked	
Approved	

STR. A' ARKANSAS RIVER TULSA COUNTY  
**GENERAL ELEV. AND PLAN**  
 19-102'-0" PRESTRESSED CONC. BEAM SPANS  
 56'-0" CLR. RDY. W/ CONC. PARAPET



FILE NO.	STATE	J/P NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
6	OKLA	07313(04)	1994	54	111

REVISIONS	DATE

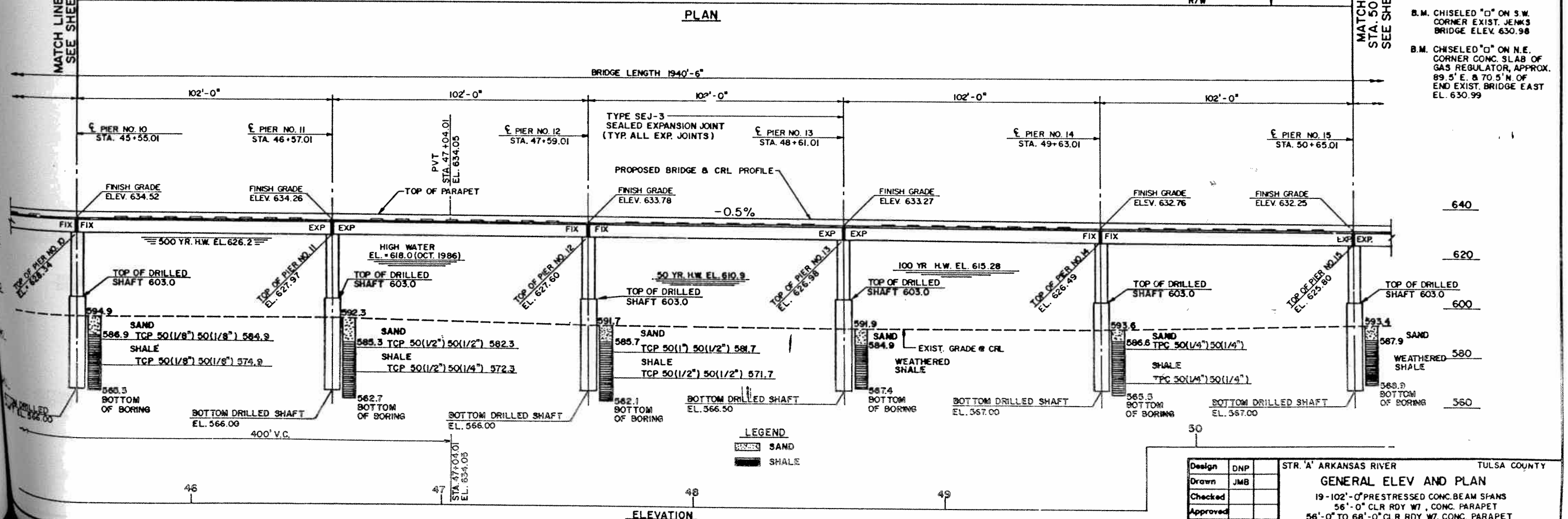
**HYDRAULIC DATA**

Q500 = 480,000 cfs  
V500 = 6.2 fpm  
H.W. EL. (500 YR.) = 626.20

Q100 = 170,000 cfs  
V100 = 7.22 fpm  
H.W. EL. (100 YR.) = 615.28

Q50 = 105,000 cfs  
V50 = 5.93 fpm  
H.W. EL. (50 YR.) = 610.90

**DRAINAGE AREA:**  
TOTAL: 74,615 SQ. MI.  
UNCONTROLLED: 22,621 SQ. MI.  
PIER SCOUR DEPTH (CALC.) = 10.4 FT  
EXTREME HIGH WATER - N/A



**PLAN**

**EXISTING BRIDGE**

**PROPOSED BRIDGE & CRL**

**STATIONING:** STA. 50+00.48, STA. 51+69.72, STA. 52+80.72, STA. 54+74.98

**HYDRAULIC DATA:**

Flow (cfs)	Velocity (fps)	H.W. EL. (500 YR)
Q500 = 480,000 cfs	V500 = 6.2 fps	H.W. EL. (500 YR) = 626.20
Q100 = 170,000 cfs	V100 = 7.22 fps	H.W. EL. (100 YR) = 615.28
Q50 = 105,000 cfs	V50 = 5.93 fps	H.W. EL. (50 YR) = 610.90

**DRAINAGE AREA:**

TOTAL: 74,615 SQ. MI.  
 UNCONTROLLED: 22,621 SQ. MI.  
 PIER SCOUR DEPTH (CALC.) = 10.4 FT.  
 EXTREME HIGH WATER - N/A



Design	DNP	STR "A" ARKANSAS RIVER	TULSA COUNTY
Drawn	SGG	GENERAL ELEV AND PLAN	
Checked		19 - 102'-0" PRESTRESSED CONC. BEAM SPANS	
Approved		68'-0" CLR RDY W7, CONC. PARAPET	
Squad		STATE J/P NO. 07313(04) Sheet No. 52	

Attachment E  
Locations of 1980 Borings Advanced by W.R. Holway &  
Associates, Tulsa River Parks Authority at Zink Lake Dam

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# TULSA RIVER PARKS AUTHORITY

## LOW WATER DAM PROJECT

A PROJECT OF THE TULSA URBAN RENEWAL AUTHORITY



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C-2 GENERAL SITE PLAN  
C-3 FOUNDATION BORINGS

C-4 ARKANSAS RIVER DISCHARGE HYDROGRAPHS 11th STREET BRIDGE 1964 TO 1973 - GAUGE NO. 07164500 (RECORDED FLOWS)  
C-5 ARKANSAS RIVER DISCHARGE HYDROGRAPHS 11th STREET BRIDGE 1974 TO 1978 - GAUGE NO. 07164500 (RECORDED FLOWS)

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C-7 AREA PLAN "B" & DETAILS  
C-8 AREA PLAN "C" & DETAILS  
C-9 AREA PLAN "D" & DETAILS  
C-10 AREA PLAN "E" & DETAILS  
C-11 AREA PLAN "F" & DETAILS  
C-12 AREA PLAN "G" & DETAILS  
C-13 AREA PLAN "H" & DETAILS  
C-14 AREA PLAN "J" & DETAILS

WATER LINE  
GAS LINE  
SANITARY SEWER  
ELECTRICAL

W  
G  
S  
E

### LEGEND

ELEVATION MARK  
DETAIL OR SECTION REFERENCE  
GRID WORK POINTS

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C-17 PUMP PIT PLAN, SECTIONS, & DETAILS  
C-18 BOARD WALK SECTION & DETAILS  
C-19 AREA "E" SECTIONS & DETAILS  
C-20 AREA "F" WALL SECTIONS  
C-21 AREA "F" SECTIONS & DETAILS  
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C-23 OGEE WEIR SECTIONS  
C-24 OGEE WEIR SECTION  
C-25 BASCULE GATE PLANS & SECTIONS  
C-26 BASCULE GATE PIER SECTIONS & EAST GATE HOUSE SECTIONS  
C-27 SECTIONS THROUGH EAST & WEST BASCULE GATES  
C-28 WEST BANK SECTIONS & DETAILS AT AREA PLAN "J"  
C-29 RETAINING WALL COUNTERFORT TYPE "A"  
C-30 RETAINING WALL COUNTERFORT TYPE "B"  
C-31 RETAINING WALL COUNTERFORT TYPE "C"  
C-32 RESTROOM FACILITY

1" = 70' N  
3" = 20' W  
C-10  
W.P.

C-33 RESTROOM FACILITY REINFORCING & DETAILS  
C-34 OBSERVATION PLATFORM ROOF FRAMING PLANS, SECTIONS, & DETAILS  
C-35 NORTH & SOUTH STAIR PLANS, SECTIONS & DETAILS  
C-36 FOUNTAIN PLAN & DETAILS  
C-37 ART AND GRAPHICS PLANS AND DETAILS  
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C-40 FOUNTAIN PLAN  
C-41 FOUNTAIN ELEVATION 612  
C-42 FOUNTAIN ELEVATION 614  
C-43 FOUNTAIN ELEVATION 616  
C-44 FOUNTAIN ELEVATION 618  
C-45 FOUNTAIN ELEVATION 620  
C-46 FOUNTAIN ELEVATION 622 & 624  
C-47 FOUNTAIN ELEVATION 626 & 628, SECTIONS  
C-48 FOUNTAIN ELEVATIONS  
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M-2 FOUNTAIN MECHANICAL PLANS & DETAILS  
M-3 FOUNTAIN MECHANICAL PLAN  
E-1 GENERAL LIGHTING & POWER PLAN  
E-2 ELECTRICAL SCHEDULES & DETAILS  
E-3 ELECTRICAL DETAILS

VOID

**W. R. HOLWAY AND ASSOCIATES**  
4111 SOUTH DARLINGTON STREET  
TULSA, OKLAHOMA 74135

*Do Not Remove From Office*

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TULSA URBAN RENEWAL AUTHORITY

TULSA RIVER PARKS AUTHORITY  
LOW WATER DAM PROJECT

COVER AND INDEX OF DRAWING

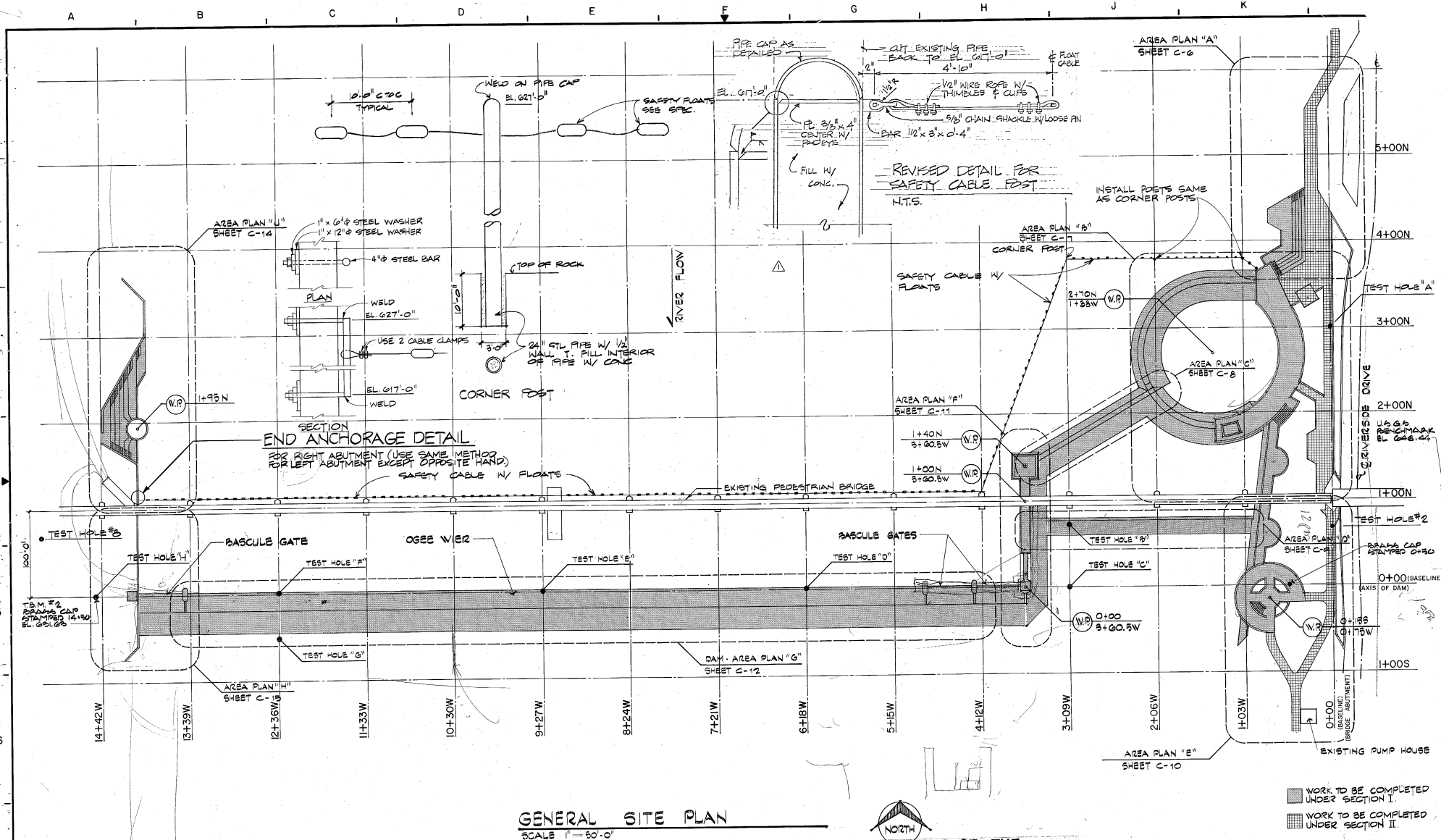
REVISION	DATE	BY	DATE
1	6/23/82	TH	JM
2			
3			



W. R. Holway and Associates  
Consulting Engineers - Tulsa, Oklahoma







REVISED SAFETY CABLE POST	HA	JM	11/19/82
RECORD DRAWING	TM	JM	6/23/82
REVISION	DATE	DATE	DATE

DRAWN	J.C.W.	BY	DATE
CHECKED	J.M.	DATE	DATE
SUPV.	ENR.	DATE	DATE
PROJ.	MGR.	DATE	DATE



**W. R. Holway and Associates**  
Consulting Engineers - Tulsa, Oklahoma

A PROJECT OF THE  
**TULSA URBAN RENEWAL AUTHORITY**  
**LOW WATER DAM PROJECT**

**RECORD DRAWING**

GENERAL SITE PLAN

NOTED  
REV.

